





MISCELLANEOUS PAPER S-71-17

EARTHQUAKE RESISTANCE OF EARTH AND ROCK-FILL DAMS

Report 5

PERMANENT DISPLACEMENTS OF EARTH EMBANKMENTS
BY NEWMARK SLIDING BLOCK ANALYSIS

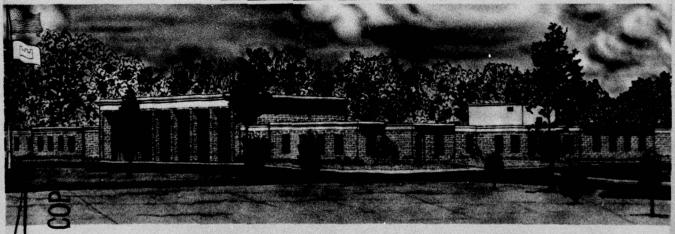
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November 1977 Report 5 of a Series

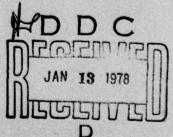
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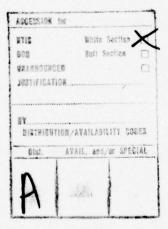
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20. ABSTRACT (Continued).

digitized accelerograms and compute the permanent displacements from the velocity-time history and the resistance coefficients. All records were scaled to 0.5g peak acceleration and 30-in./sec peak velocity, and the resulting scaled permanent displacements are called standardized maximum displacements. A total of 169 horizontal and 10 vertical corrected accelerograms were processed in addition to several synthetic records.

The greatest standardized maximum displacements, computed from records of the magnitude-6.5 San Fernando earthquake of 9 February 1971 on soil sites, were about 1.5 times above Newmark's upper bound, while those for all other earthquakes analyzed were near or below Newmark's upper bound. The maximum values computed from the Jennings et al. synthetic record for a magnitude 8+ earthquake were about 1.7 times higher than Newmark's upper bound. Those for the Seed-Idriss synthetic record fell slightly below those for the Jennings et al. synthetic records. Ten records from rock sites compared with 47 records from soil sites indicate that permanent displacements on rock sites are about 75 percent of those on soil sites from earthquakes of the same magnitude, peak acceleration, and peak velocity. It was found that standardized maximum displacements were roughly proportional to the duration of shaking, and consequently were positively correlated with earthquake magnitude.

Appendixes A and B list the earthquakes and the ground motion data used, respectively. Appendix C presents data on the synthetic records.





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PREFACE

This report is part of ongoing work at the U. S. Army Engineer Waterways Experiment Station (WES) in the Civil Works Program, "Earthquake Resistance of Earth and Rock-fill Dams," CWIS No. 31144, sponsored by the Office, Chief of Engineers, U. S. Army. This report was prepared by Dr. Arley G. Franklin and Mr. Frank K. Chang of the Earthquake Engineering and Vibrations Division, Soils and Pavements Laboratory (S&PL), under the general direction of Mr. James P. Sale, Chief, S&PL, and Dr. Francis G. McLean, Chief, Earthquake Engineering and Vibrations Division.

Directors of WES during the period of this study were COL G. H. Hilt, CE, and COL John L. Cannon, CE. Technical Director was Mr. F. R. Brown.

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CONVERSION FACTORS, U. S. CUSTOMARY TO METRIC (SI) UNITS OF MEASUREMENT

U. S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	By	To Obtain
inches	2.54	centimetres
inches per second	2.54	centimetres per second

EARTHQUAKE RESISTANCE OF EARTH AND ROCK-FILL DAMS

PERMANENT DISPLACEMENTS OF EARTH EMBANKMENTS BY NEWMARK SLIDING BLOCK ANALYSIS

PART I: INTRODUCTION

- 1. In his 1965 Rankine Lecture, "Effects of Earthquakes on Dams and Embankments," Newmark described simple concepts for computing the displacement of a sliding mass in an embankment subjected to earthquake accelerations. He also presented charts, based on a sliding block model, for estimating the upper bounds of potential permanent displacements due to an earthquake with a given peak acceleration and peak particle velocity. The calculations from which these charts were derived were based on ground motions from four earthquake accelerograms. Since

 Newmark's 1965 lecture, the Parkfield earthquake, with 0.5g recorded on the San Andreas Fault, and the San Fernando earthquake, with 1.25g recorded in the epicentral region, have occurred, and a large number of strong-motion accelerograms have been collected from these and other events. It was decided to use these records to extend the data base for Newmark's charts.
- 2. Newmark presented charts for the cases of symmetrical and non-symmetrical resistance to sliding. The case of symmetrical resistance can be of only infrequent occurrence, and leads to limited permanent deformations. It was judged to be of minor interest, and only the second case, that of a sliding block moving downslope, was dealt with in this study.

3. A total of 169 horizontal and 10 vertical strong-motion records from 27 earthquakes and 10 synthetic accelerograms were used with the sliding block analysis, and the results are presented in Part III. Listings of the earthquakes and the ground motion data used are given in Appendixes A and B, respectively, and Appendix C presents data on the synthetic records.

PART II: METHOD OF ANALYSIS

Concepts of Newmark's Method

- 4. A case of potential sliding of a portion of an embankment under earthquake loading is illustrated in Figure 1. The effective force resulting from the critical earthquake acceleration is the force NW in Figure 1. This force is the product of the weight W* of the sliding mass and the fraction N of gravitational acceleration g that is required to reduce the factor of safety to unity. The direction of the force, defined by its angle of inclination to the horizontal, $\,\theta$, is the most critical direction, or that which results in a minimum value for N. The angle θ is normally no more than a few degrees. According to Sarma, both the factor of safety and the permanent displacements are insensitive to θ , and it can be taken as zero with little error. The value of N, the critical acceleration or resistance coefficient, can be found by means of conventional methods of stability analysis, such as Bishop's Method, the Morgenstern-Price method, etc., using appropriate undrained strength values. Various trial values of N may be used so as to find the value that makes the factor of safety equal to unity. Plane, circular, or other forms of slip surface may be considered. The method of stability analysis described by Sarma uses a slip surface of arbitrary shape and determines the value of N directly.
 - 5. The force polygon for the sliding mass is shown in Figure 2b.

^{*} For convenience, symbols and unusual abbreviations used in this report are listed and defined in the Notation (Appendix D).

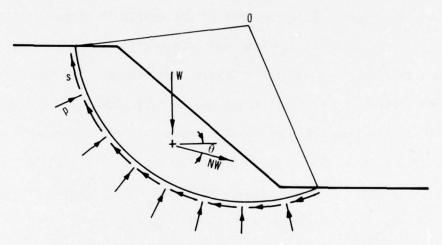
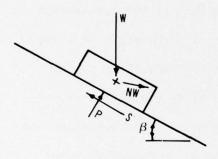
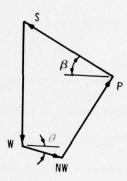


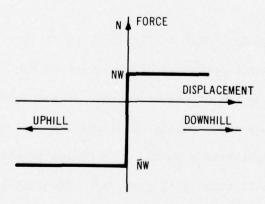
Figure 1. Potential sliding mass



a. Sliding block model



b. Force polygon for
 F.S. = 1.0

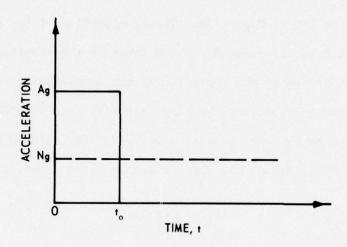


c. Force-displacement relation

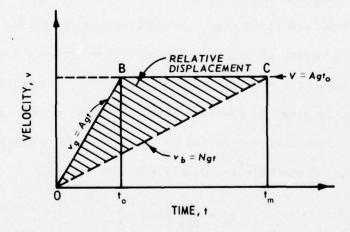
Figure 2. Mechanical model for displacement analysis

The same force polygon can also represent the forces on a rigid block that is about to slide down an inclined plane, as shown in Figure 2a. In the condition illustrated, the base is undergoing an acceleration Ng to the left and upward, the shearing resistance S has reached its limiting value, and slippage of the block relative to the plane is imminent, or, in other words, the factor of safety is unity. The force P is the resultant of the normal forces, and S the resultant of the distributed shear resistance, on the slip surface of Figure 1. The angle $\,\beta$, the inclination of the plane surface, is found as the inclination of the force S. The resistance to sliding is assumed to be rigid-plastic, as shown in the force-displacement diagram in Figure 2c. The resistance to sliding is unsymmetrical, because the block can slide downslope more easily than upslope. For the computations of permanent displacement presented in this report, it is assumed that the resistance to sliding upslope is sufficiently large that upslope sliding never occurs. This assumption results in the greatest permanent displacement, and thus represents the worst case.

6. For an embankment that suffers a slope failure due to seismic ground motions, the total permanent displacement of a sliding mass relative to the base is the sum of the increments of displacement occurring during a number of individual pulses of ground motion. Consider a single rectangular acceleration pulse, with ground acceleration Ag lasting from time zero until time to (Figure 3a). The instantaneous velocity of the ground, which is given by



a. Rectangular acceleration pulse



b. Relative displacement due to rectangular acceleration pulse

Figure 3. Newmark's displacement concepts

$$v_g = \int Ag dt , (0 \le t \le t_0)$$

$$v_g = Agt_0 , (t \ge t_0)$$
(1)

follows the path OBC in Figure 3b. The acceleration of the sliding block is limited to the value Ng by the limit of the shearing resistance that can be mobilized at the contact. If the acceleration Ag is less than or equal to Ng, the block and the base will move together; but if Ag is greater than Ng, the absolute velocity of the sliding block follows the path OC in Figure 3b, which represents the relation

$$v_b = \int Ng dt$$
 (2)

Relative motion between the base and the block continues until both attain the same absolute velocity, which occurs at time t_m . From that time on, the base and block move together, without slippage. Since the absolute displacements of the base and block are given by the areas under their respective velocity versus time curves, the relative displacement, u_m , is given by the area between the two curves, the triangle OBC, which is shown hachured in Figure 3b. From the geometry of the diagram, the value of the relative displacement is given by

$$u_{m} = \frac{V^{2}}{2gN} \left(1 - \frac{N}{A} \right) \tag{3}$$

where V is the maximum ground or base velocity, which is equal to Agto. If nothing happens to produce further relative motion, or reverse it, the relative displacement will be permanent, and will thus be called permanent displacement.

Computation of Permanent Displacements

- 7. The computation of the permanent displacement, u_m , from an earthquake record can be visualized from the plot shown in Figure 4. A plot of this type can also be used to perform the computation graphically. The curve $v_g(t)$ represents the ground or base velocity (the velocity of the ground beneath the sliding mass), while the critical acceleration for the sliding mass is represented by a slope, dv/dt = Ng, on the velocity versus time plot. Wherever the ground acceleration (slope of the ground velocity curve) exceeds the critical acceleration, the velocity curve of the sliding mass departs from that of the ground and follows a linear path, $v_b = Ngt$, until the two velocities again become equal, at which time relative movement ceases. The total permanent displacement, u_m , is then given by the sum of the areas between the two velocity curves.
- 8. In Newmark's 1965 Rankine Lecture, results were presented for scaled permanent displacements computed from four strong-motion records which were available at that time. The four earthquake records were first scaled to a maximum acceleration of 0.5g and a maximum ground velocity of 30 in./sec* by adjusting the acceleration and time scales. The resulting scaled values of relative displacement, called standardized maximum displacements, were plotted against the ratio N/A on a logarithmic plot, and upper bound curves were proposed for various ranges in the value of N/A.

^{*} A table of factors for converting U. S. customary units to metric (SI) units of measurement is found on page 4.

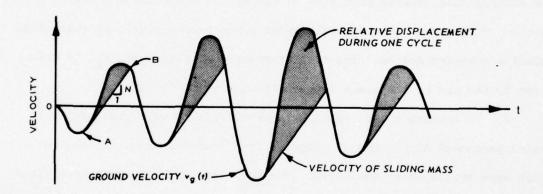
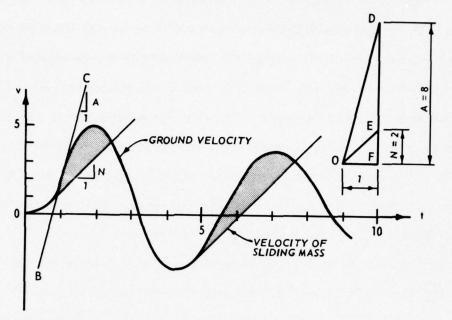


Figure 4. Computation of permanent displacement--unsymmetrical resistance

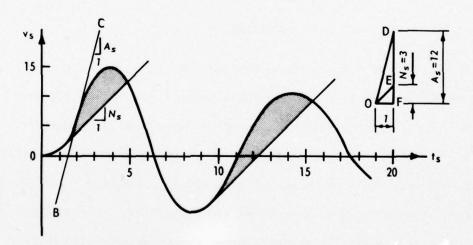
9. Since 1965, many additional strong-motion records have become available. The study reported herein was made to determine whether these additional records materially affect the upper bounds for permanent displacement proposed by Newmark. For this study, the ground velocities, ground displacements, and permanent displacements were computed numerically, using the trapezoidal rule, by means of a simple computer program written in Fortran IV for the G. E./Honeywell 635 digital computer. The ground motion records used were 179 digitized, baseline-corrected accelerograms of the California Institute of Technology (CIT) Volume II series. The four earthquake records used by Newmark were included. Agreement in computed permanent displacements for these records was close, but not exact, probably because of some differences in the form of the earthquake records used.

Scaling

- 10. All of the strong-motion records used were scaled to obtain a maximum ground acceleration of 0.5g and a maximum ground velocity of 30 in./sec, in order to obtain results of the same form as Newmark's.
- 11. The purpose of the scaling of the earthquake records is to permit direct comparison of permanent displacements computed from records with a wide range of peak accelerations and velocities. The process can be illustrated graphically with a hypothetical example including numerical values, as shown in Figure 5. Suppose that a portion of an earthquake record is represented by a velocity versus time plot (which is normally obtained by integration of the acceleration record) as shown in Figure 5a, and that the peak acceleration has been identified by a



a. Hypothetical velocity record at natural scale



b. Hypothetical velocity record scaled to $v_{\rm g}/v$ = 3 and $A_{\rm g}/A$ = 1.5

Figure 5. Hypothetical velocity records, showing scaling to arbitrary peak acceleration and peak velocity values, for A/N = 4

tangent BC to the velocity curve at the point of maximum slope. It can be seen that the peak velocity V is represented as 5, and the peak acceleration A as 8. (The values have been chosen for numerical convenience and simplicity, rather than realism, and units of measurement have been dispensed with). Suppose also that the geometric construction has been made on the record for the computation of the permanent displacement of a sliding mass whose critical acceleration N is 2; the ratio A/N is thus 4. The relation between these acceleration values is also illustrated by the diagram ODEF at the right-hand side of the figure. In the diagram, if the base OF of the triangle represents unit time, then the altitude DF, measured along the velocity axis, represents the peak acceleration A, and the altitude EF represents the critical acceleration N. Just as previously described for Figure 4, the shaded areas between the curves represent increments of permanent displacement of the sliding mass relative to the ground or base, and the sum of these increments is the total permanent displacement, um.

- 12. Scaling this record to arbitrarily chosen standard values of peak velocity and acceleration is done by adjusting the accelerations and the time scale; however, it is equivalent to performing the following two operations:
 - a. Transforming the ordinate (velocity axis) by scaling it so that the highest peak on the velocity curve corresponds to the desired peak velocity. The value chosen for the example is 15 (see Figure 5b).

b. Transforming the abscissa (time axis) by scaling it so that the slope of the line representing the peak acceleration has the desired value. In the example, a peak acceleration of 12 was chosen. In other words, the acceleration diagram is scaled so that the distance DF equals 12 units on the new velocity axis; the distance OF then represents one time unit.

Another way of looking at this scaling is to note that it is dimensionally correct to write a velocity as the product of an acceleration and a time, or

$$v = at$$
 (4)

Therefore,

$$\frac{V_{S}}{V} = \frac{A_{S}}{A} \cdot \frac{t_{S}}{t}$$
 (5)

which gives

$$\frac{t_s}{t} = \frac{V_s}{V} \cdot \frac{A}{A_s} \tag{6}$$

in which the subscript s denotes scaled values. For the example, the required time scaling is

$$\frac{t_s}{t} = \frac{15}{5} \cdot \frac{8}{12} = 2 \tag{7}$$

- 13. The resulting transformed velocity record, as shown in Figure 5b, is identical with the original except for the scaling of the coordinate axes, and examination of the figure will show that the desired relationships among accelerations, velocities, and displacements are all present. Note particularly that in the transformation of the peak acceleration A to a scaled peak acceleration A_s , the critical acceleration of the sliding mass, N, is scaled in the same proportion, so that the ratio N_s/A_s is the same as N/A.
- 14. The relationship between the permanent displacement u_m and its representation on the scaled plot, which is shown as u_s , is apparent from a comparison of Figures 5a and 5b. The scale relationship between the areas is equal to the product of the horizontal and vertical linear scales; thus,

$$\frac{u_{m}}{u_{s}} = \frac{V}{V_{s}} \cdot \frac{t}{t_{s}} \tag{8}$$

Substituting for the time scaling the expression derived earlier,

$$\frac{t_s}{t} = \frac{V_s}{V} \cdot \frac{A}{A_s} \tag{9}$$

gives

$$\frac{u_{m}}{u_{s}} = \frac{V^{2}A_{s}}{V_{s}^{2}A} \tag{10}$$

For the relation between the standardized maximum displacement $u_{_{\rm S}}$ and the unscaled permanent displacement $u_{_{\rm M}}$, $V_{_{\rm S}}$ = 30 in./sec and $A_{_{\rm S}}$ = 0.5g are used, which gives

$$u_{m} = u_{s} \cdot \frac{v^{2}(0.5g)}{(30)^{2}Ag}$$

$$= u_{s} \cdot \frac{v^{2}}{1800A}$$
(11)

where V is the maximum ground velocity, in inches per second; A is the maximum ground acceleration, as a fraction of g, in the unscaled record; and $u_{\rm g}$ and $u_{\rm m}$ are in inches.

PART III: RESULTS

- 15. Representative results from the analysis of a total of 169 horizontal and 10 vertical accelerograms from 27 strong earthquake events of the western United States are plotted in Figures 6 through 10, and discussed in the following sections. In addition, computations were made for the Jennings et al. (CIT) and Seed-Idriss synthetic accelerograms, and for a synthetic record developed to fit the Nuclear Regulatory Commission Regulatory Guide 1.60 spectra. Total displacement was also correlated with Richter magnitude, duration, and distance.
- 16. Figures 6 through 10 show the standardized maximum displacement, u_s , versus the value of $\frac{N}{A}$ (where A and N are as previously defined) for about half of the earthquake records analyzed, and include those that yielded the highest values of displacement. Figure 6 shows results from 9 accelerograms of the Kern County, California, earthquake of 21 July 1952, at distances of 43 to 126 km and at soil sites. Figure 7 contains the results from 47 accelerograms of the San Fernando earthquake of 9 February 1971 at distances of 22.4 to 185 km, at soil sites. Figure 8 presents the results of 15 records of western United States earthquakes of magnitudes M 5.2 to 6.0, at soil sites. Figure 9 represents 10 vertical components of the 1971 San Fernando earthquake. Figure 10 represents 10 records of various western United States earthquakes at rock sites. To permit comparisons with the records not shown in these plots, Appendix B lists the values of standardized maximum displacement for three values of N/A for all records analyzed.

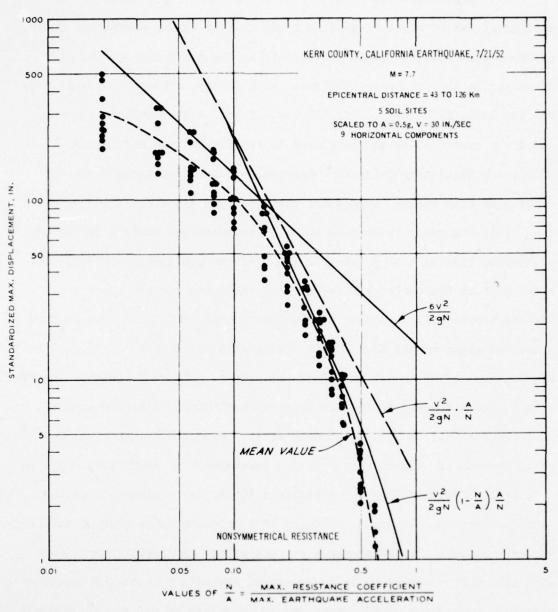


Figure 6. Permanent displacements due to Kern County earthquake, 21 July 1952 (soil sites)

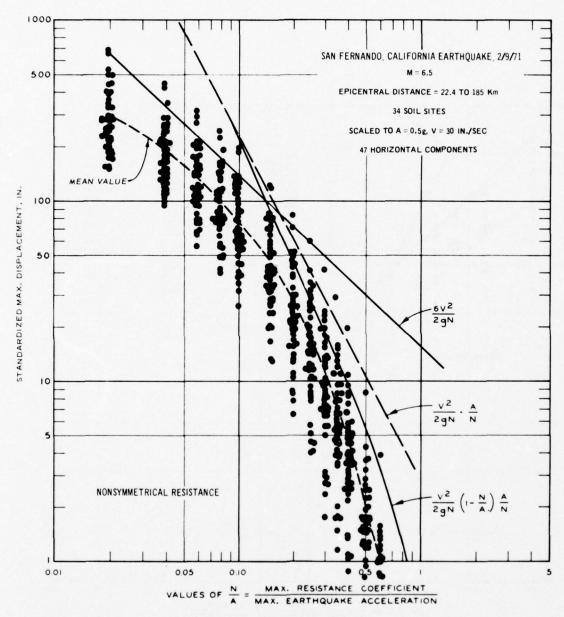


Figure 7. Permanent displacements due to San Fernando earthquake, 9 February 1971 (soil sites)

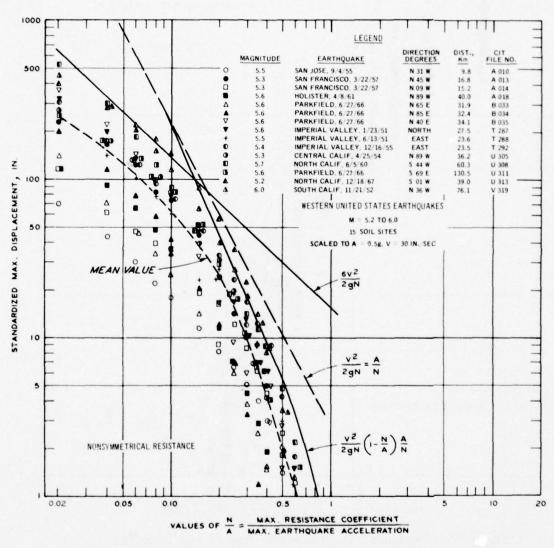


Figure 8. Permanent displacements due to western United States earthquakes of magnitudes 5.2 to 6.0

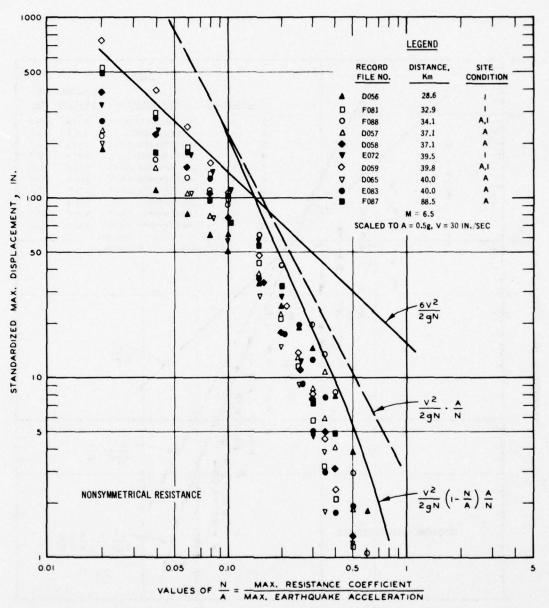


Figure 9. Permanent displacement due to San Fernando earthquake, 9 February 1971 (alluvial and intermediate sites), computed from vertical component records

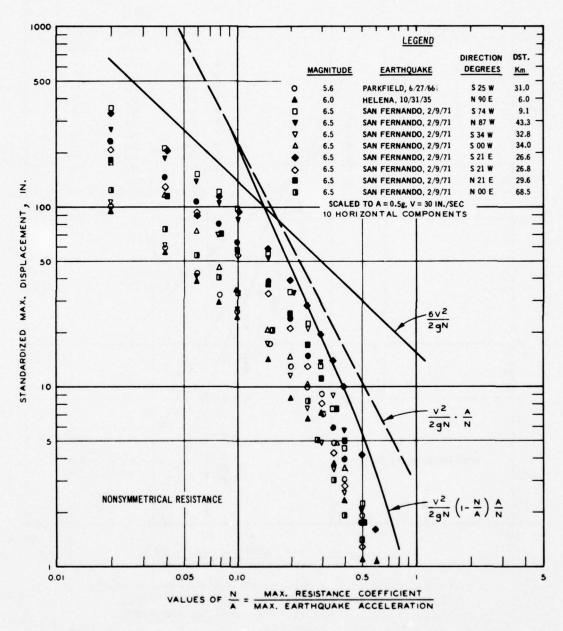


Figure 10. Permanent displacements computed from rock site accelerograms

Upper Bounds of Permanent Displacements

- 17. Figures 6 through 10 show, in addition to the values of us versus N/A, three curves chosen by Newmark to represent upper bounds for us as computed from the four earthquake records used in his 1965 paper. In Figures 6 through 9, there are several points, the highest belonging to the 1971 San Fernando earthquake, lying above Newmark's upper bound curves. It can be seen from these figures that in order to envelope the permanent displacements computed from the present data, the bounding curves must be raised.
- 18. Figure 10, while based on only 10 records from three earthquakes, suggests that permanent displacements at rock sites will be appreciably lower than at soil sites, for earthquakes of equal magnitude and peak motion values, and for all of the values shown are conservatively bounded by Newmark's upper two curves.

Correlation with Magnitude and Duration

19. The computed values of standardized maximum displacement, when plotted against duration of shaking, as shown in Figure 11 for the soil site records of the San Fernando earthquake, can be seen to be approximately proportional to the duration. The duration for this purpose was considered to be the period lasting until the last acceleration peak with at least 0.25 times the peak acceleration. Plots of values from other earthquake records (not shown here) are similar. Because duration of shaking correlates positively with earthquake magnitude, the standardized maximum displacement values can also be expected to increase with magnitude. This tendency is illustrated in Figure 12, in which mean value curves

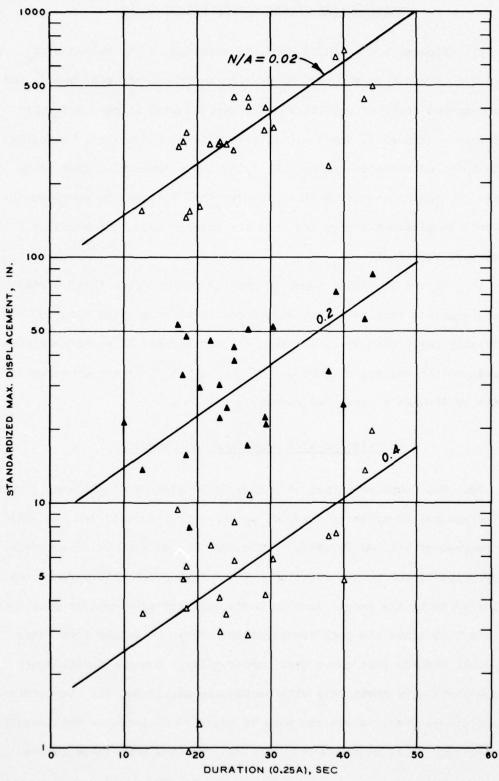


Figure 11. Permanent displacement versus duration, San Fernando earthquake, 9 February 1971 (soil sites)

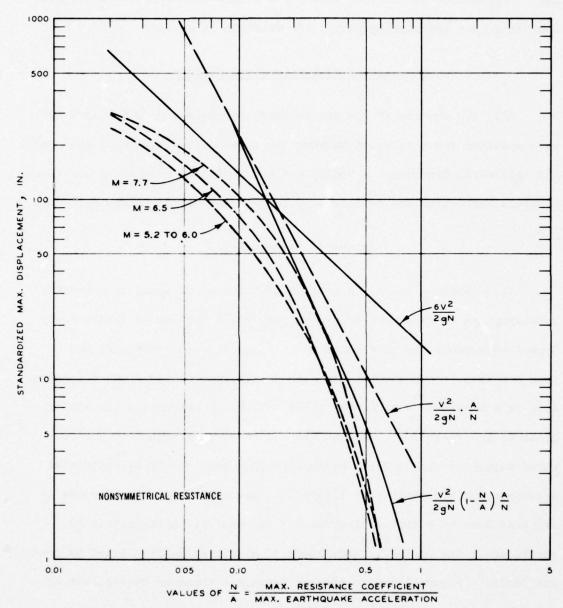


Figure 12. Mean permanent displacement for different magnitudes of earthquakes (soil sites)

for the earthquakes of Figures 6, 7, and 8 have been plotted. The systematic variation with magnitude, as reflected by the mean values, is small compared to the scatter band for a single earthquake, however; and for N/A values approaching unity, the relation is obscure.

Correlation with Epicentral Distance

20. The records of the San Fernando earthquake of 9 February 1971 were examined for a relation between the standardized maximum displacement and epicentral distance. A weakly defined positive correlation was found, probably reflecting the dominance of long-period motion in the far field.

Synthetic Earthquakes

- 21. Jennings et al. generated four different types of synthetic accelerograms to represent ground motions for a variety of earthquakes. Type A represents the accelerations in a magnitude 8 earthquake and Type B motion is expected with magnitude 7. Type C is for the epicentral area of a magnitude 5 or 6 earthquake and Type D represents the motion close to the fault for a shallow earthquake of magnitude 4 or 5. Computed values of standardized maximum displacement for these artificial accelerograms are plotted in Figure 13. Newmark's upper bound curve is exceeded here by a factor of about 1.7 for the Type A (magnitude 8) earthquake. The synthetic earthquake of magnitude 8-1/4 modeled by Seed and Idriss (Figure 14) also exceeds Newmark's bounding curves, but by a lesser amount for most values of N/A.
- 22. The standardized maximum displacements obtained from a synthetic accelerogram developed to fit the response spectra given in the Nuclear

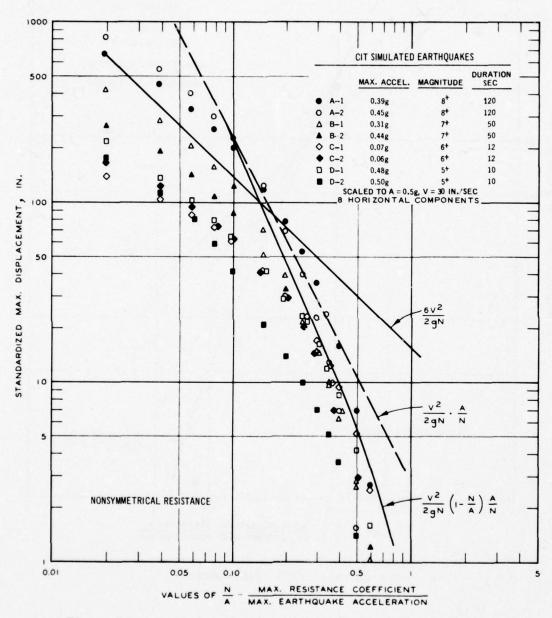


Figure 13. Permanent displacements due to CIT simulated earthquakes by Jennings et al. $^{\rm L}$

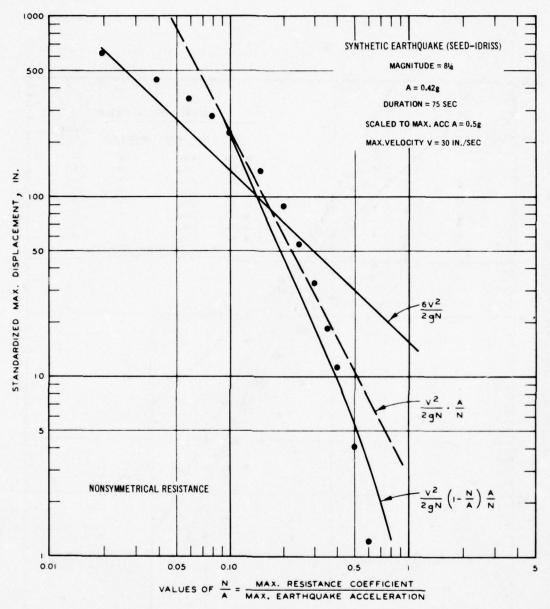


Figure 14. Permanent displacements due to synthetic earthquake of magnitude 8-1/4 (Seed-Idriss5)

Regulatory Commission Regulatory Guide 1.60⁶ are shown in Figure 15. The curve is close to the average curve of the San Fernando earthquake of magnitude 6.5 on rock sites, as shown in Figure 10, but falls far below Newmark's limiting curves and the higher values computed in this study.

23. Upper bound curves for all natural and synthetic earthquake records analyzed in the present study are shown in Figure 16.

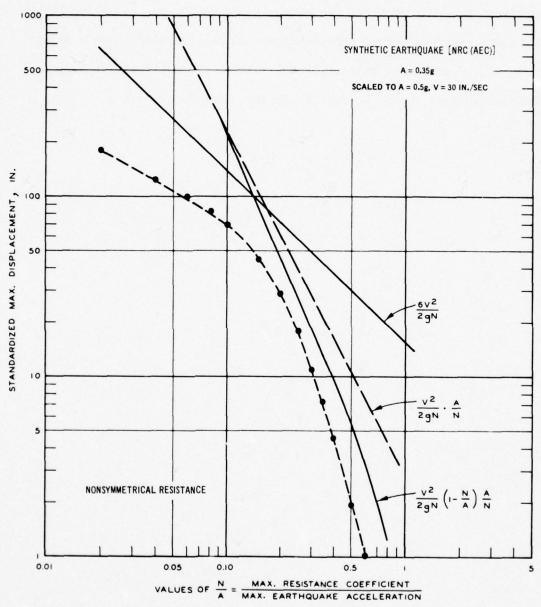


Figure 15. Permanent displacements due to synthetic earthquake corresponding to response spectrum of U. S. Nuclear Regulatory Commission (NRC) Regulatory Guide 1.606

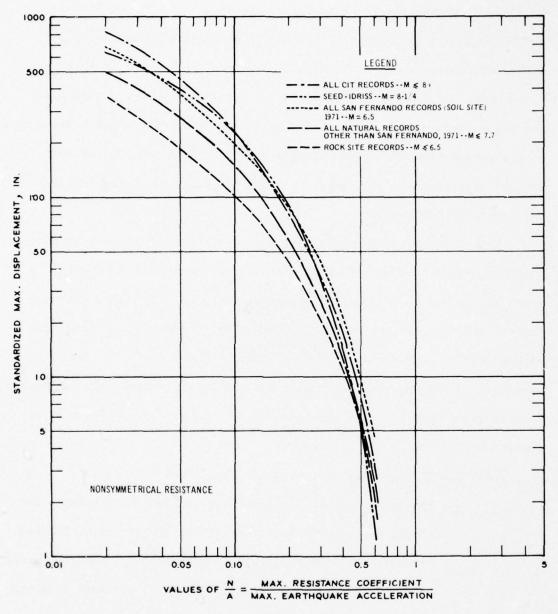


Figure 16. Upper bound envelope curves of permanent displacements for all natural and synthetic records analyzed

PART IV: SUMMARY AND CONCLUSIONS

- 24. In 1965, Newmark presented the results of calculations of scaled permanent displacements (standardized maximum displacements) of earth embankments under earthquake loading, on the basis of a sliding block model and four earthquake records. Upper bound curves were given for the purpose of earth and rock-fill dam design. Since that time, many more strong-motion earthquake records have been obtained, and it was decided to extend the data base for Newmark's plots using the available new strong-motion data.
- 25. A total of 169 horizontal and 10 vertical strong-motion earthquake records of the western United States were scaled to 0.5g peak acceleration and 30-in./sec peak velocity and processed with a computer program written for this study. Additionally, the synthetic earthquake records of Jennings et al. and Seed-Idriss, and a synthetic record developed to fit the response spectra of the Nuclear Regulatory Commission Regulatory Guide 1.60 were processed. Only the case of nonsymmetrical resistance to sliding was considered.
 - 26. The findings of this study are summarized as follows:
 - a. New upper bounds of standardized maximum displacement for actual earthquakes were established by records of the San Fernando earthquake of 1971 (magnitude 6.5), which produced values about 1.5 times higher than those obtained from the four earthquake records used in 1965 by Newmark.
 - <u>b</u>. The greatest standardized maximum displacements found in this study were produced by the Jennings et al. and Seed-Idriss synthetic earthquakes of magnitude 8+, and were

- about 1.7 times higher than Newmark's upper bounds.
- On the basis of comparison of 10 records from rock sites with 47 from soil sites, computed permanent displacements at rock sites are about 75 percent of those at soil sites for earthquakes of equal magnitude, peak acceleration, and peak velocity.
- <u>d</u>. Standardized maximum displacement was found to be proportional to the duration of shaking, and consequently to be positively correlated with magnitude, but the trend is weak and considerable scatter exists.

REFERENCES

- Newmark, N. M., "Effects of Earthquakes on Dams and Embankments," Geotechnique, Vol 15, No. 2, Jan 1965.
- 2. Sarma, S. K., "Seismic Stability of Earth Dams and Embankments," Geotechnique, Vol 25, No. 4, 1975, pp 743-761.
- 3. California Institute of Technology, Earthquake Engineering Research Laboratory, "Strong-Motion Earthquake Accelerograms; Corrected Accelerograms and Integrated Ground Velocities and Displacements," Vol II, Parts A-N, 1971-1975, Pasadena, Calif.
- 4. Jennings, P. C., Housner, G. W., and Tsai, N. C., "Simulated Earthquake Motions," Earthquake Engineering Research Laboratory Report, 1968, California Institute of Technology.
- 5. Seed, H. B. and Idriss, I. M., "Rock Motion Accelerograms for High Magnitude Earthquakes," EERC 69-7, 1969, Earthquake Engineering Research Center, University of California, Berkeley, Calif.
- 6. U. S. Nuclear Regulatory Commission (formerly Atomic Energy Commission), Regulatory Guide 1.60, Revision 1, 1973.

APPENDIX A: LIST OF EARTHQUAKE EVENTS

Table Al

List of Earthquake Events

- Maximum Intensity	900000	m00	11991	81811	117778
Magni- tude	6.57	9.01	0.07.00 6.07.00	5.57 5.57 5.57 5.57	5.6
Depth	16.0	16.0	16.0	16.0	16.0
Long.	117 58 00 118 08 00 124 36 00 115 30 00 111 58 00	111 58 00 112 00 00 111 58 00 125 15 00 115 35 00	115 13 00 115 10 00 124 48 00 115 30 00 125 24 00	119 35 00 124 36 00 118 15 00 116 00 00 121 18 00	122 42 00 115 44 00 124 48 00 119 01 00 118 39 00
Lat	33 37 00 33 47 00 41 42 00 32 15 00 46 37 00	16 37 00 16 36 00 16 37 00 10 30 00 32 53 00	32 54 00 32 15 00 40 18 00 32 44 00 40 42 00	34 22 00 40 36 00 33 47 00 32 58 00 37 06 00	47 06 00 32 59 00 40 17 00 35 00 00 35 17 00
Time	PST PST PST MST	MST MST MST PST PST	PST PST PST PST	PST PST PST PST	PST PST PDT PDT
Time	1754 0110 1449 0552 1138	1218 2058 0742 2042 0825	1842 0435 2210 2037 0145	2351 0813 0042 0822 0429	1156 2317 2011 0453
Date	10 Mar 1933 2 Oct 1933 6 Jul 1934 30 Dec 1934 31 Oct 1935	31 Oct 1935 21 Nov 1935 28 Nov 1935 6 Feb 1937 12 Apr 1938	5 Jun 1938 6 Jun 1938 11 Sep 1938 18 May 1940 9 Feb 1941	30 Jun 1941 3 Oct 1941 14 Nov 1941 21 Oct 1942 9 Mar 1949	13 Apr 1949 23 Jan 1951 7 Oct 1951 21 Jul 1952 23 Jul 1952
Earthquake Area	Long Beach, Calif. Southern California Eureka, Calif. Lower California Helena, Mont.	Helena, Mont. Helena, Mont. Helena, Mont. Humboldt Bay, Calif. Imperial Valley, Calif.	Imperial Valley, Calif. Imperial Valley, Calif. Northwest California Imperial Valley, Calif. Northwest California	Santa Barbara, Calif. Northern California Torrance-Gardena, Calif. Borrego Valley, Calif. Northern California	Western Washington Imperial Valley, Calif. Northwest California Kern County, Calif. Kern County, Calif.
No.	M t w o t	9 6 9 10 10 10 10 10 10 10 10 10 10 10 10 10	12217	16 17 19 20	12 8 25 4 X

(Sheet 1 of 3)

Table Al (Continued)

Maximum Intensity	8-	~~~!!	-1195	~ v v v v v	16867	2 of 3)
Magni- M tude In		3.45.66 9.085.0	3.74	5.00 4.5.0	5.007.	(Sheet
Depth	16.0	16.0 16.0 16.0	16.0 16.0 13.8	11111	11.0	
Long.	124 25 00 121 10 00 115 43 00 119 01 00 121 48 00	116 00 00 123 52 00 121 47 00 115 30 00 115 30 00	115 30 00 115 54 00 115 54 00 119 13 12 122 28 00	122 29 00 122 27 00 122 29 00 121 26 00 124 53 00	121 18 00 124 12 C0 122 18 00 118 31 18 120 29 54	
Lat o ' " N	40 12 00 35 50 00 32 57 00 35 00 00 36 48 00	31 30 00 40 47 00 37 22 00 33 00 00 33 00 00	33 00 00 31 42 00 31 42 00 34 07 06 37 40 00	37 40 00 37 39 00 37 39 00 36 47 00 40 49 00	36 30 00 40 58 00 47 24 00 34 29 06 35 57 18	
Time	PDT PST PST PST	PST PST PST PST	PST PST PST PST	PST PST PST PST	323 PST 917 PST 729 PST 346 PST 026 PST Continued	
Time	0441 2346 2017 1534 1233	0427 1156 1801 2117 2142	2207 0633 0725 1056 1048	1144 1515 1627 1926 1718	2323 0917 0729 2346 2026	
Date	22 Sep 1952 21 Nov 1952 13 Jun 1953 12 Jan 1954 25 Apr 1954	12 Nov 1954 21 Dec 1954 4 Sep 1955 16 Dec 1955 16 Dec 1955	16 Dec 1955 9 Feb 1956 9 Feb 1956 18 Mar 1957 22 Mar 1957	22 Mar 1957 22 Mar 1957 22 Mar 1957 19 Jan 1960 5 Jun 1960	8 Apr 1961 4 Sep 1962 29 Apr 1965 15 Jul 1965 27 Jun 1966	
Earthquake Area	Northern California Southern California Imperial Valley, Calif. Wheeler Ridge, Calif. Central California	Lower California Eureka, Calif. San Jose, Calif. Imperial County, Calif. Imperial County, Calif.	Imperial County, Calif. El Alamo, Baja, Calif. El Alamo, Baja, Calif. Southern California San Francisco, Calif.	San Francisco, Calif. San Francisco, Calif. San Francisco, Calif. Central California	Hollister, Calif. Northern California Puget Sound, Wash. Southern California Parkfield, Calif.	
No.	26 27 29 30 30	33 33 34 35 35	36 33 39 40	th th th th th	74 148 149 20	

Table Al (Concluded)

No.	Earthquake Area	Date	Time	Time	Lat o	2	Long.	Depth	Magni- tude	Maximum Intensity
17.		Aug	0936	PST	148	0.0	30	16.0	6.3	9
22 %	Northern California	Dec]	0407	PST	30 %	0 0	36 96	11	. o . o . o	~ 9
57	Northern California Borrego Mtn, Calif.	18 Dec 1967 8 Apr 1968	0925	PST	37 00 3 33 11 2	36	121 47 18	11.11	6.5	9 2
56	Lytle Creek, Calif. San Fernando, Calif.	12 Sep 1970 9 Feb 1971	0630	PST	34 16 12 34 24 42	QI QI	117 32 24 118 24 00	8.0	5.4	۲a

APPENDIX B: STRONG-MOTION DATA, EARTHQUAKES OF WESTERN UNITED STATES, UNIFORMLY PROCESSED AT CALIFORNIA INSTITUTE OF TECHNOLOGY

Table 81 Strong-Motion Data, Earthquakes of Western United States, Uniformly Processed at California Institute of Technology

Here of the interest is a series of the control of	CIT File No.	Recording Station	Site Classifi-	Date of Earthquake	Epicenter	Instrument	A Peak Acceleration cm/sec2	V Peak Velocity cm/sec	D Peak Displace- ment	A D	Epicentral Distance km	Richter Magnitude M	Modified Mercalli Intensity	Duration	3800	us , in., for N/A	N/A
Particle California Particle Particl	A001	El Centro Site, Imperial Valley	4	5-18-40	32° th· N 115° 27° W	S 900 E	341.7		19.8	8.8.8	9.3	6.7	VIII	30	230.9	55.6	1.90
Material Particle Mate	A002	Morthwest California Earth- quake, Ferndale City Hall	H	10-7-51		200	102.0	7.4	2.7	10.6 5.4 8.7	56.3		>				
Part Control Extension A 7-21-50 1370 N 812 1537 1547 542 1450	A003	Kern County Earthquake, Athenaeum	4	7-21-52		88	% 5.5 % 3.1 % 3.3	6.5	3.00.0	1.8	126.0	7.7	VII	20		100.5	4.4
Control Sub-flowing, such as the control Sub-flowing Su	A0014	Mern County Earthquake, Taft Lincoln School	*	7-21-52		380	152.7 175.9 102.9	15.7	5.00	4.2 5.2 11.5	43.0	7.7	VII	₹.		85.3	2.09
State Stat	A005	Kern County Earthquake, Santa Barbara Courthouse	*E	7-21-52		ж о 5 8 8 8 8 8	87.8 128.6 43.6	19.3	5.8	3.0.8	89.5	7.7	VII	18	366.2	136.2	3.10
Figure County setting Figure Fi	A006	Kern County Earthquake, Hollywood Storage Basement	4	7-21-52		88	54.1 43.5 22.5	6.1 9.4 4.2	5.9	2.9	119.5	7.7	VII	82 82	214.7	150.7	3.94
Particle Designate, Directed 1 12-21-54 13	A007	Kern County Earthquake, Hollywood Storage F. E. Lot	æ	7-21-52			86.1 20.3 20.3	3.0	3.4	3.3	119.5	7.7	VII	92	230.8	145.7	2.51
State Surthquake, State 12-21-54 38-34 N N Lab 2 155.7 25.6 14.2 1.7 1.7 1.2 1.7 1.7 1.5	A008	Eureka Earthquake, Eureka Federal Building	н	12-21-54		N 11° W N 79° E Up	164.5 252.7 81.3	31.6 29.4 8.2	12.4 14.1 4.7	2.0	24.0	6.5	VII	30.56	157.9	33.4	2.70
One Description of Surface Sand of Surface Sand of Surface Sand One Surface Sand of Surface Sand One Surface Surfac	4009	Sureka Earthquake, Ferndale City Hall	н	12-21-54		26	155.7 197.3 41.9	35.6	14.2 9.6 3.9	2.8	7.07	6.5	VII	500	167.7	53.0	3.20
El Almon, Barla, Continual	A010	San Jose Earthquake, San Jose Bank of America Basement	¥	65-4-6		310	105.8	10.8	2.8	2.4 6.8	9.6	5.5	ии	30	4.07	17.7	1.39
El Alamo, Baja, California 2-9-56 125.9	A011	El Alamo, Baja, California Earthquake, El Centro Site, Imperial Valley Irrigation District	4	2-9-56		88	32.4 50.1 12.4	2.9	2.4 4.1 1.6	6.24	125.9	6.8	ľ	70		145.8	2.91
San Francisco Earthquake, I 3-22-57 37°40'N N 10°5 E 6.0 16.8 5.3 VII 26 228.1 73.8 San Francisco Pacific E 7.3 Principal San Francisco Pacific E	AOLZ	El Alamo, Baja, California Earthquake, El Centro Site, Imperial Valley Irrigation District (Aftershock)		2-9-56		88	15.4	1.9	2.93	3.3	125.9						
Sun Francisco Earthquike, 1 3-22-57 37°40' N N N O°5 W 41.8 2.9 1.3 6.5 15.2 5.3 VII 25 114.5 33.5 Estimation All Annahr 1.2 1.2 1.0 1.0 1.0 1.0 Estimation for Annahr 1.2 1.2 1.0 1.0 1.0 Estimation for Annahr 1.2 1.2 1.0 1.0 1.0 Estimation for Annahr 1.2 1.2 1.2 1.0 1.0 1.0 Estimation for Annahr 1.2 1.2 1.2 1.0 1.0 1.0 Estimation for Annahr 1.2 1.2 1.2 1.0 Estimation for Annahr 1.2 1.2 1.2 1.0 Estimation for Annahr 1.2 1.2 1.2 1.0 Estimation for Annahr 1.2 1.2 1.0 Estimation for Annahr 1.2 1.2 1.0 Estimation for Annahr 1.2 1.2 Estimation for Annahr 1.2 Estimation for An	A013	San Francisco Earthquake, San Francisco Pacific	н	3-22-57		120	26.99 86.89		1.1	6.0 2.5 10.7	16.8	5.3	VII	56	228.1	13.8	2.05
San Francisco Euritquake, I 3-22-57 37°40'N N 10°E 91.8 4.9 2.3 7.8 11.8 5.3 VII 12 125.8 23.6 5.8 N Francisco Enterpolation Contribution Contribu	A014	San Francisco Earthquake, San Francisco Alexander Building, Basement	н	3-22-57		810	41.8 45.4 30.0		1.3	6.5	15.2	5.3	VII	53	114.5	33.5	2.51
Sun Francisco Entiquido, 1 3-22-57 37% of 8 89.8 51 11 3.5 14.6 5.3 80.0 80.0 3.1 14.6 5.3 80.0 80.0 3.1 14.6 5.3 80.0 80.0 3.1 14.6 5.3 80.0 80.0 80.0 80.0 80.0 80.0 80.0 80	A015	San Francisco Earthquake, San Francisco Golden Gate Park	н	3-22-57		900	81.8 102.8 37.2		0.00	3.9	11.8	5.3	IIA	12	125.8	1 1 23.6	1.07
	A016	San Francisco Sarthquake, San Francisco State Building Basement	н	3-22-57		810	83.8 55.1 43.5		0.0	3.5	14.6	5.3	VII				

Note: Locations in California unless otherwise noted.

A = alluvium, I = intermediate, and HR = hard rock.

	for N/A	1 0.5		5 0.65	3 0.88	3 0.88	99.0 9			1.94	8 1.08				1 0.50			0.89	2 0.44
	u fn.	0.02 0.1 G		164.7 36.5	151.9 39.3	151.9 39.3	115.7 22.6			7.721 5.795	94.6 23.8				582.9 127.1			382.2 83.0	138.8 25.2
		Duration		30	09	30	30			36	\$				98			8	1,1
	Modified	Mercalli Intensity	ıv	II	Y.	VI	M	>	>	14	VII	IA.	I,	VIII	иш	м	IIA	II.	VII
	Richter	Magnitude M	5.3	5.6	6.5	6.5	6.3	5.4	5.4	6.5	6.0	5.5	9.9	1.1	1.7	5.5	65	5.9	9.6
	Epicentral	Distance	24.3	0.04	69.8	109.9	47.8	38.2	38.2	60.8	9.9	55.3	1.86	57.8	16.8	43.2	43.0	61.1	31.9
	A D	102	14.6 18.1 24.5	9.6.9	2.9	8.53 6.63 6.63	9.2	6.25	6.4 6.6 6.6	5.0	3.8	12.6 3.2 9.7	10.0	4.60.60	0.6.6	24.6	3.2 5.7 17.9	5.7 4.6 11.3	2.1
Deak	Displace-	ment	1.5	0.000	3.9	1.3 4.1	15.5	8.4.0	0.0	3.50	3.7	9.0	2.50	2.5	10.1	066	1.1	3.8	26.3 4.3
		Velocity cm/sec		17.1	25.8 14.7 3.4	6.0	17.0	23.61	0.80	20.5	13.3	6.6	3.5	9.2	21.4 17.0 6.8	6.9	9.68	8.0 3.0	14.1
4	Peak	Acceleration cm/sec2	39.0 23.7	63.4 175.7 19.1	127.8 56.3 29.7	28.9	130.6	5.5% 6.4.80	32.1 36.1 10.7	156.8 179.1 68.1	143.5 142.5 87.5	140.9 87.1 31.6	61.3 38.4 19.2	66.5 65.9 22.0	161.6 274.6 90.6	53.1 74.1 29.2	63.9 66.8 35.5	134.2 194.3 59.9	202.2
		Instrument		S 01° W N 89° W	3 900 s 3 900 du	00 N 00 00 00 00 00 00 00 00 00 00 00 00	M 08 08 09 04 04 04 04 04 04 04 04 04 04 04 04 04	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	N N 90° N U 90° E E	N N 80 02 03 03 03 03 03 03 03 03 03 03 03 03 03	S 45° W	S 145° W W 455° W	S 02° W W 88° W	3 04° E 3 86° W Up	S LLC W	N 21° E S 69° E Up	s 016 € S 86 ₩ Up	N 65° E Down
		Epicenter	37° 40° N 122° 29° W	36 40' N 121°18' W	33°09' и 116°08' и	33°09' N 116°08' W	33°35' N 117°59' W	33°47' N 118°08' W	33° 47° W	32°12° N 115°30° W	111° 58° W	124° 18° W	125°24° W	120° te° w	122° 12' W	120°25' W	35°00' N 119°01' W	122° 18° W	35° 54° N 120° 54° W
	Date	Earthquake	3-22-57	19-8-1	4-8-68	89-8-4	3-10-33	10-2-33	10-2-33	12-30-34	10-31-35	9-11-38	2-9-41	4-13-49	4-13-49	9-22-52	1-12-54	4-59-62	99-12-99
	Site	Classifi-	н	*	*	4	*	*	4	4	£	н	H	4	4	H	≪	4	4
		Recording Station	San Francisco Earthquake. Oakland City Hall Basement	Holister Earthquake, Holister City Hall	Borrego Mt Earthquake, El Centro Site, Imperial Valley Irrigation District	Borrego Mt Barthquake, San Diego Light & Power Building	Long Beach Earthquake, Vernon CMD Building	Southern California Earth- quake, Hollywood Storage Building Penthouse	Southern California Earth- quake, Hollywood Storage Building Basement	Lower California Earthquake, El Centro Imperial Valley	Helena, Montana Sarthquake, Helena, Montana, Carroll College	lst Northwest California Earthquake, Ferndale City Hall	2nd Northwest California Earthquake, Ferndale City Hall	Western Washington Earth- quake, District Engineers Office at Army Base	Western Mashington Earth- quake, Olympia, Mashing- ton, Highway Test Laboratory	Northern California Earth- quake, Ferndale City Hall	Wheeler Ridge, California Earthquake, Taft Lincoln School Tunnel	Puget Sound, Mashington Earthquake, Olympia, Washington, Highway Test Laboratory	Parkfield, California Earthquake, Cholame, Shandon Array No. 2
	CIT	No.	A017	A018	4019	A020	1208	8022	8023	8024	8025	3026	1208	8028	6208	9030	8031	8032	B033

(Continued)

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CIT Fills	Recording Station	Site Classifi- cation	Date of Earthquake	Epicenter	Instrument	A Peak Acceleration cm/sec2	V Peak Velocity cm/sec	Peak Displace- ment cm	V ² D	Epicentral Distance km	Richter Magnitude M	Modified Mercalli Intensity	Duration	. s . 0	us , in., for N/A	N/A 0.5
8034	Parkfield, California Earthquake, Cholame, Shandon Array No. 5	₹	99-12-9	35° 54° W	N 050 K N 850 E Down	347.8 125.7 116.9	22.5	5.2	3.6	32.4	5.6	VI	23	198.7	1.9	0.02
8035	Parkfield, California Earthquake, Cholame, Shandon Array No. 8	*	99-12-9	35° 54° N	N 50° N N DOWN N TOWN	232.6	10.8	4 6 6 6 6 7 6 7 6 7 6 7 6 7 6 7 6 7 6 7	8.8 7.6 8.0	34.1	5.6	r,	20	363.0	1 69 1	1.45
9608	Parkfield, California Earthquake, Cholame, Shandon Array No. 12	4	99-12-9	35° 54" ¥	N 50° E N 40° W Down	52.1 63.2 44.6	5.00	5.7	4.64	36.5	5.6	I,		323.4	62.5	0.05
B037	Parkfield, California Earthquake, Temblor No. 2	E .	99-12-9	35° 54° N 120° 54° W	N 65° W S 25° W Down	264.3 340.8 129.8	22.5 4.4	5.5	9.5.4	31.0	9.6	IIA	23	100.6	26.3	0.73
8038	Parkfield, California Earthquake, San Luis Obispo Recreation Building		99-12-99	35°54' N 120°54' W	8 54° w 95° w 90° w	14.2 6.1 6.1	1.1	0.0	14.1	76.1	5.6	٨		569.3	101.4	1.32
9039	2nd Northern California Earthquake, Eureka Federal Building	н	12-10-67	124°36° W	S 11° E N 79° E Down	20.1 19.5 7.7	2.8	1.1	3.5	90.6	5.8	ь		102.6	19.0	1.02
Bollo	Borrego Mountain Earth- quake, San Onofre SCE Power Plant	н	4-8-68	33°09' и 116°08' и	N 33° E N 57° W Down	45.5 54.2	3.50	2.9	7.5	134.4	6.5	۸		7.672	1971	0.16
00/1	San Fernando Earthquake, Pacoima Dam	Æ	2-9-71	34°24' N 118°23'42" W	S 16° E S 74° W Down	1148.1 1054.9 696.0	113.2 57.7 58.3	37.7	440	9.1	9.9	×	16	162.1 346.9	32.2	2.19
2000	San Fernando Earthquake, Aftershock at 22.6 sec. Pacoima Dam		2-9-71	34°24' N 118°23'42" W	S 74° W S 16° E Down	27.1 20.7 8.2	1.5	1.7	10.2							
1700	San Fernando Earthquake, Affershock at 104.6 sec. Pacoima Dam		2-9-71	34°24' N 118°23'42" W	S 74° W S 16° E Down	109.9	1,44	0.00	10.5							
8700	San Fernando Earthquake, 8244 Orion Boulevard, lat Floor, Holiday Inn	*	2-9-71	34°24' W 118°23'42" W	N 000 W S 90° W Down	250.0 131.7 167.5	30.0	14.9 13.8 14.6	3.2	22.4	9.9	VII	7	398.0	126.1	1.26
1500	San Fernando Barthquake, 250 Bast First Street, Basement, Los Angeles	∢.	2-9-71	34°24' W	N 36° E N 54° W Down	97.8 122.7 18.0	17.1 21.9 7.8	9.2 5.8 5.8	23.7	42.8	9.9	VII	15	172.0	188.1	1.47
7500	San Fernando Barthquake, 445 Figueroa Street, Sub- basement, Los Angeles	1,4	2-9-11	34°24" W 118°23"42" W	N 52° W S 38° W Down	117.0	17.4	11.8	2.3	41.9	9.9	VII	007	215.9	58.0	2.88
9500	San Fernando Earthquake, Old Hidge Foute, Castaic	н	2-9-71	34°24' F 118°23.7' W	и 21° Е и 69° и Вомп	309.4 265.4 153.3	27.2	3.6.8	14.0	28.6	9.9	I,	818	223.3	50.7	3.86
	San Fernando Earthquake, Hollywood Storage Basement	4	2-9-11	34°24' и 118°23.7' и	N 300 E M	103.8	17.0 19.4 6.0	8.6 13.1 3.8	5.2	37.1	9.9	VII	000	210.1 284.2 236.6	62.0 87.7 63.3	1.55
8500	San Fernando Earthquake, Hollywood Storage P. E. Lot	٧	2-6-2	34°24' N 118°23.7' W	N 900 E W	167.3 207.0 87.0	21.1	8.0 14.7 3.0	4.9 6.8 10.4	37.1	9.9	VII	22.22	382.2	53.3	1.28
6500	San Fernando Earthquake, 19 - Avenue, The Stars Subbusement	4	2-9-71	34°24' N 118°23.7' W	N 176° W S 414° W Down	133.8 147.1 66.7	16.7	2.5	10.9	39.8	9.9	VII	55	479.9	85.6	0.99

11		31.5	Date			Peak	Peak	D Pesk Displace-		Epicentral	Richter	Modified				
F11e	Recording Station	cation	of Earthquake	Epicenter	Instrument	Acceleration cm/sec2	Velocity cm/sec	ment	127	Distance	Magnitude M	Mercalli Intensity	Duration	0.02	0.02 0.1 C	0.5
D062	San Fernando Earthquake, 1640 South Marengo Street, 1st Floor, Los Angeles	٧.	2-9-11	34°24' N 118°23.7' W	8 52° 8 50 52° 8	130.0	16.1		20.00	42.8	9.9	VII	30	231.2	63.9	2.77
5900	San Fernando Earthquake, 3710 Wilshire Boulevard, Dasement, Los Angeles	A.1	2-9-71	34°24' N 118°23.7' W	\$ 900 s	155.7	22.1 9.0	12.9 4.9	⊏	40.0		VII	77 77	155.9	34.7	0.39
8900	San Fernando Earthquake, 7080 Hollywood Bonlevard, Basement, Los Angeles	4	2-6-21	34°24' N 118°23.7' W	N N 900 N 900 E E E	98.0	13.3 5.63 5.63	4.2	1.07	35.0	9.9	VII				
EOTI	San Fernando Earthquake, Wheeler Ridge	*	2-6-2	34°24' N 118°23.7' W	300 00 N 000 00 N 000 00 N 000 00 N 000 00	26.5	6.6.6	3.3	10.2 8.5 7.4	96.0	9.9	>		259.5	33.1	1.08
E072	San Fernando Earthquake, 4680 Wilshire Boulevard, Basement, Los Angeles	н	2-6-21	34°25.7' W	N 75° W N 15° E Down	82.2 115.0 64.8	20.8 21.5 6.9	114.7	8.6.3	39.5	9.9	MI	188	329.1	110.6	0.64
E075	San Fernando Earthquake, 3470 Wilshire Boulevard, Subbasement, Los Angeles	₹	2-9-71	31°24' N 118°23.7' W	N 000 E S 90° W Down	133.8 111.8 17.3	22.3 18.5 7.3	4.11.6 3.9	48.6	40.1	9.9	II.	83	207.2	8,11	2.17
8078	San Fernando Earthquake, Water and Power Building, Basement, Los Angeles	H	2-9-11	34°24' N 118°23.7' W	N 500 N S 400 N	126.5 169.2 67.2	23.2 16.1 10.2	13.7 8.9 6.4	2.85	42.5	9.9	VII	7.7	179.5	36.2	2.18
1906	San Fernando Earthquake, Santa Felicia Dam, Outlet Works	1	2-6-2	34°24' N	S 92° E Down	213.0 198.3 63.7	6.5.4	2.60	23.7 8.8	32.9	6.6	Ħ	***	505.5	63.2 96.4	3.27
E082	San Fernando Earthquake, Santa Felicia Dam, Crest		2-9-71	34°24° N 118°23.7° W	S 15° E S 75° W Down	203.3 174.0 65.0	22.2 18.1 6.2	2.53		32.8	9.9	TA.	37	274.6	18.1	4.19
8083	San Fernando Earthquake, 3407 6th Street, Basement, Los Angeles	۷.	2-6-2	34°24' N 118°23.7' W	S 00° W N 90° E Down	158.2 161.9 55.5	18.3 16.5 8.8	9.0 10.3	6.1	40.0	9.9	VII	52.52	228.3	72.7	1.75
9904	San Fernando Farthquake, Vernon, CMD Building	∢	2-6-2	34°24' N 118°23.7' W	N 83° W S OT° W	104.6 80.5 12.7	17.4	10.7	3.8.6	7.67	9.9	>		306.8	62.2	2.36
1904	San Fernando Earthquake, Engineering Building, Santa Ana, Orange County	¥	2-6-2	34°24' N 118°23.7' W	8 04° 29 8 86° 14 Up	26.8 28.2 16.7	0.004	3.6	2.5	88.5	9.9	Ķ		269.7	17.6	0.65
P088	San Fernando Earthquake, 633 East Broadway, Munic- ipal Service Building, Glendale	A,1	2-9-71	34°24' N 118°23.7' W	S 20° E Down	265.7 209.1 131.5	30.7 23.5 15.6	11.1 5.3 5.6	3.00	34.1	9.9	VII	221	221.0	117.5	2.95
6804	San Fernando Earthquake, 808 South Olive Street, Los Angeles	4	2-6-2	34°24' N 118°23.7' W	S 53° E S 37° ¥ Down	131.9 139.0 75.3	20.8	11.5	4.8.E	44.0	9.9	II	22.23	181.4	1 1 1	0.18
2604	San Fernando Earthquake, 2011 Zonal Avenue, Base- ment, Los Angeles	ы	17-6-2	34°24′ K 118°23.7′ W	S 62° E	64.2 79.1	13.8	10.3	3.9.5	43.1	9.9	MI				
5604	San Fernando Earthquake, 120 North Robertson Boulevard, Subbasement, Los Angeles	4	2-9-11	34°24' N 118°23.7' W	3 88° E S 02° ¥ Down	8.88	16.8 17.9 6.2	3.9	3.5	37.4	9.9	VII				
8604	San Fernando Earthquake, 646 South Olive Avenue, Basement, Los Angeles	æ	17-6-5	34°24' N 118°23.7' W	S 53° E S 37° W Down	236.4 192.0 69.2	21.8 18.5 9.6	13.2 13.4 5.3	4.0	42.7	9.9	II.	8	218.9	118	0.14
							(Continued)								(Sheet	(Sheet 4 of 12)

N/A	0.5	5.71	0.71	2.95	1.21	1.49	4.07	16.0	0.09		1.99	2.51	1.20	1.85	2.64	1.27		0.34
u. , in., for N/A	0.1	11.0	11 33.5	161.0	36.7	11.55.8	13.9	18.1	110.9		32:1	110.1	7:11	130.0	4.11	1 82.5		11 38.7
3	0.02	178.2	122.6	605.1	180.1	338.5	204.1	257.9	540.7		154.8	287.2	351.6	318.8	253.1	287.1		244.7
	Sec						52	56	35	23	45	30	86	92	72	r.		897
Modified	Mercalli Intensity	Λ	>	>	>	VII	VII	VII	VII	VII	VII	IA	VII	VI	VII	I		IA
Richter	Magnitude	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9		9.9
Epicentral	Distance	107.6	68.5	4.54	52.2	38.7	36.1	39.8	39.8	31.5	40.5	32.3	29.3	50.2	41.1	16.8		38.2
		6.6 8.1 12.2	10.0 8.5 7.6	9.9	2.9	5.3 9.6	5.0	3.8	5.6	5.8	6.0.0 0.0.0	3.6	2.7	3.1	5.50	3.8 5.3 5.3	2.8.2	F 6.3
D Peak Displace-	ment cm	1111	0.00	2.5	2.3	0 6 6 6	2.3	5.3.0	2.5	0.6.9	9.2	2.73	10.4	3.98	3.4.8 3.4.4	2.1	8.18	2.3
Peak	Velocity cm/sec	2001	11.1	2.3.2.2	80 E		5.8	14.3	26.3	13.4	15.7	14.0	28.2	9.11 6.9	17.1	28.8	13.2	17.2 14.1 4.5
Peak	Acceleration cm/sec2	30.0	24.6 20.6 15.3	91.5	85.2 103.1 35.5	83.1 77.6 67.1	87.5 188.6 83.5	93.5 107.3 92.9	198.0 181.6 91.2	207.8 139.0 126.3	101.9 78.5 53.2	110.8 136.2 86.6	220.6	32.7	119.4 112.3 79.2	34.9	90.9 91.6 36.4	184.3 160.6 37.2
	Instrument	S 00° W N 90° E	N 90° E Down Down	N 90° E N 90° W Down	N 90° E Down	S 00° W N 90° E Up	8 906 8 8 906 8 Down	M M 900 M M M M M M M M M M M M M M M M	N 900 N N 900 N N 900 N N N 900 N N N 900 N N N N	S 82° E S 08° E Down	N 32° N N 38° E Down	S 50° E S 30° W Down	N 11° E N 79° W Down	S 45 E	3 3 00 00 00 00 00 00 00 00 00 00 00 00	3 900 S	N 000 E S 900 W	N 50° E N 10° W Down
	Epicenter	34°24. N 118°23.7' W	34°24. N	34°24' N 118°23.7' W	34°24. N	34°24' N 118°23.7' W	34°24'42" N	34° 24° 12" N 118° 24° 00" W	34°24'00" W	34°24.42" N 118°24.00" W	34°24'12" N 118°24'00" W	34°24.12" N 118°24.00" W	34°24'12" N	34°24'42" N	34°24'42" N	34°24'12" N 118°24'00" W	34°24.12" N 118°24.00" W	34°24'42" N 118°24'00" W
Date	Sarthquake	2-9-71	2-9-71	2-9-71	17-6-6	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	17-6-5	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71
31.0	Classifi-	4	Ei .	4	H	4	Æ	<	*	Α,1	4	4	4	«K	«	*	*	*
	Recording Station	San Fernando Earthquake, Edison Company, Colton	San Fernando Earthquake, Fort Tejon, Tejon	San Fernando Barthquake, Fumping Plant, Pear- blossom	San Fernando Earthquake, Oso Pumping Plant, Gorman	San fernando Earthquake, UCLA Reactor Laboratory, Los Angeles	San Fernando Earthquake, CIT Seismological Labo- ratory, Pasadena	San Fernando Earthquake, Athenaeum, CIT	San Fernando Earthquake, CII Milikan Library	San Fernando Barthquake, CIT Jet Fropulsion Labo- ratory Basement	San Fernando Barthquake, 611 West Sixth Street, Basement, Los Angeles	San Pernando Barthquake, Palmdale Fire Station Storage Room, Palmdale	San Pernando Earthquake, 19250 Ventura Boulevard, Basement	San Fernando Earthquake, 8639 Lincoln Avenue, Basement, Los Angeles	San Fernando Barthquake, 900 South Fremont Avenue, Basement, Albambra	San Fernando Earthquake, 2600 Mutwood Avenue, Base- ment, Fullerton	San Fernando Earthquake, 435 North Oakhurst Avenue, Basement, Beverly Hills	San Fernando Earthquake, 450 North Roxbury Drive, 1st Floor, Beverly Hills
CIT	No.	FIOI	F100	F103	F104	F105	9010	7010	61108	0110	2112	41119	81115	H118	H121	HIZL	1128	1131

(Sheet 5 of 12)

. N/A		0.56	1.76	1.29	4.25	94.9	3.23	2.25	3,79	0.36	1.19	2.02	0.34	3.07	0.88	6.79
, in., for		122.7	11.3	1.45	93.2	72.9	97.3	1 55.1	167	24.3	7.61	1 39:5	120.3	11 18.2	<u>4</u> 11	128.3
0.02		285.7	183.3	204.5	331.8	254.9	227.7	185.0	166.2	152.4	183.0	144.9	403.6	234.9	290.8	440.3
Duration		39	22	37	27	55 55	07	19	56	25	33	ឡ	56	50	56	07
Modified Mercalli Intensity	VII	IIA	17	VI.	I,	IA	VII	IIA	IIA	۸	VII	VI	>	>	>	٨
Richter Magnitude M	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9
Epicentral Distance km	38.9	29.0	59.6	26.8	56.6	23.3	34.9	39.9	30.8	139.8	42.9	70.7	84.3	70.8	70.8	75.6
V2 D	900	8 8 8 4	2.0	5.83	10.4	2.9	2.5	3.9	6.3.3	9.4.9	2.3	12.0	3.0.0	6.73	4.89	10.5
D Peak Displace- ment cm	H.3	2.8.1	400	1.57	040	3.98	17.5 15.3 7.0	3.4.3	6.4.4	2.0.2	13.7	1.000	200 d 200 d	464	0.00	1.5
V Peak Velocity cm/sec		16.1 22.3 7.9	14.0	2000 2000	9.4.0	12.4	31.5 28.8 18.1	17.5	15.0	2.8	20.9	2.6	3.9	889	2.9 2.9 1.8	6.4.9 6.6.8
A Peak Acceleration on/sec2	88.3 6.5.3	140.2 129.0 99.9	108.9	168.2 143.5 150.8	119.3	346.2 277.9 105.3	113.9 103.4 106.4	107.6	164.2	15.9	83.4 116.0 41.6	20.8 16.7 38.5	23.9 29.9 18.2	25.7	13.1 57.2 24.7	67.3 67.3 41.5
Instrument	N 54° E 8 36° E Down	S 81° E S 09° W Down	N 21° E S 69° E Down	S 69° E S 21° W Down	N 21° E N 69° W Down	N 21° E N 69° W Down	3 00 s 2 900 s 2 900 mmod	N 000 N 200 D 200	N 000 N S 900 N Down	N 33° E N 57° W Down	N 37° E S 53° E Down	8 00° K N 90° E Down	3 90° w 5 90° w 5 90° w	N 650 W N 250 E Down	N 65% E S 25% E Down	3 50° E 3 10° E 5 10° E
Epicenter Location	34°24°12" N 118°24°00" ¥	34°24'12" %	34°24°16" N 118°24°00" W	34°24'12" N 118°24'00" W	34°24.12" N 118°24.00" W	34°24°12" N 118°24°00" W	34°24' N 118°23'42" W	34°24' N 118°23'42" W	34°24" W	34°24" W	316°23°12" W	34°24' N 118°23'42" W	34024. W	34,54° N 118° 23° 42° W	34°24' N 118°23'42" W	312 24.00" W
Date of Earthquake	2-9-71	2-9-71	2-6-21	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-6-71
Site Classifi- cation	4	4	EH.	H	Ħ	н	4	A,I		H	4	H	V.	н	H	H
Recording Station	San Fernando Emrthquake, 1800 Century Fark East, Basement (FG), Los Angeles	San Fernando Barthquake, 15910 Ventura Boulevard, Basement, Los Angeles	Sen Fernando Barthquake, Lake Hughes Array No. 1	San Fernando Earthquake, Lake Hughes Array No. 4	San Fernando Sarthquake, Lake Hughes Array No. 9	San Pernando Barthquake, Lake Hughes Array No. 12	San Fernando Earthquake, 15107 Van Gwen Street, Basement, Los Angeles	San Fernando Barthquake, 616 South Normandie Ave- nue, Basement, Los Angeles	San Fernando Earthquake, 3838 Lankershim Boulevard, Basement, Los Angeles	San Fernando Earthquake, Nuclear Power Flant, San Omofre	San Fernando Earthquake, 1150 South Hill Street, Subbasement, Los Angeles	San Fernando Earthquake, Tehachapi Pumping Plant, CWR Site, Grapevine	San Fernando Earthquake, 4000 West Chapman Avenue, Basement, Orange	San Fernando Earthquake, 6074 Park Drive, Ground level, Wrightwood	San Fernando Earthquake, 6074 Park Delve, Ground level, Wrightwood	San Fernando Earthquake, Carbon Canyon Dam
CIT File No.	1134	1137	3141	311/2	3143	11/110	31145	8,116	9917	1711	M176	W179	M180	M183	мзви	M185

(Sheet 6 of 12)

(Continued)

		1 1																
	r N/A	0.5	1.26	8.67	0.34		0.54	1.46	1.48	4.33	1.92	2.47	3.67	1.68	3.66	1.39		
	ug , in., for N/A	0.1	1.42	11.3	11.56.4		33.3	198.8	g 1.1	128.0	3,11	1.69	123.5	118.6	102.2	1.92		
	300	0.05	405.2	1,98.0	200.5		110.9	655.2	288.0	1,46.3	174.5	291.4	281,3	275.8	151.7	103.7		
	Duration	sec	54	25 25	4.5	69	52	88	20	1,3	23	30	69	3	23	8		
	Modified	Intensity	VI	IA	VII	I.	IIA	>	124	>	II.	II.	II.	I.	14	I.	>	>
	Richter	X	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9		9.9
	Epicentral Distance	Ę	54.1	72.1	38.9	67.8	1.0.7	122.6	15.4	185.0	34.0	1,2.0	73.8	73.6	108.2	32.8		
	A D	V2	6.1 5.0 10.4	10.1	6.0	8.4.6	4.44	5.04	3.1	5.2	0.4.6	4.07	1.5	2000	5.6	440	5.69	5.8
Peak	Displace- ment	CH	5.0 5.3	0.00	10.8 5.4 2.4	3.4	7.7	1.52.1	9.6.8	1.0	7.28 5.45 3.38	9.78	5.81 7.27 3.58	6.39 8.72 2.83	1.30	1.23	2.32	1.32
٨	Peak	cm/sec	8 6 6 8 6 7 9	3.7	17.0	1000	19.5	3.6	2.6.4	2001	20.5 14.5 7.42	21.50	9.88 6.12	10.30	3.45 2.86 1.52	3.37	3.67	2.86
×	Peak	cm/sec2	99.7 88.6	75.7 75.9 28.3	114.4 126.5 62.5	24.7 40.1 18.9	883 7.6.5.	31.0	35.0	35.4	176.0	137.0 238.0 118.0	25.9	28.4 28.1 16.1	37.4 13.9 18.5	64.6 97.0 32.90	16.40	38.46
	Instrument	Component	S 37° E S 53° W Down	N 75° W N 15° E Down	N 54° E N 36° W Down	N 65° E S 25° E Down	N 29° E N 61° ¥ Down	N 57° W N 33° E Down	N 76° W S 14° W Down	N 45° E N 45° W Down	3 300 s s s 900 mmod	N 28° E N 62° ¥ Down	N 00° E N 90° E Up	N 21° W S 69° W Up	N 90° E N 90° E Down	N 35	N 15% E	8 7 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8
	Epicenter	Location	34°24.42" W	34°24'42" N 118°24'00" ¥	34°24'42" N 118°24'00" W	342 24.00" W	34°24'12" N	34°24'42" N 118°24'00" ¥	34°24'12" N 118°24'00" W	34°24'12" N	34°24'12" N 118°24'00" W	34°24'12" N 118°24'00" ¥	34°24'12" N 118°24'00" ¥	31624.12" N	34°24'12" N	34° 24' 00" W	3424.42" N	31,51,12" N 118°24'00" W
	Date	Earthquake	2-9-71	2-9-71	2-9-71	2-9-71	2-6-2	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-6-2	2-9-71	2-9-11	2-9-71
	Site Classiff.	cation	⋖	«	e¢.	н	ы	A	≪	*	a: Hi	esc.	×	44	4	S	н	<
		Recording Station	San Fernando Barthquake, Whittler Narrows Dam	San Fernando Earthquake, San Antonio Dam, Upland	San Fernando Barthquake, 1880 Century Park East, Farking, 1st level, Los Angeles	San Fernando Earthquake, 2516 Wa Tejon, Ground level, Palos Verdes Estates	San Fernando Barthquake, 2500 Wilshire Boulevard, Basement, Los Angeles	San Fernando Barthquake, San Juan Capistrano	San Fernando Earthquake, Long Beach State College, Ground level	San Fernando Estthquake, Anza Fost Office Storage Room, Anza	San Fernando Earthquake, Griffith Park Observatory, Los Angeles	San Fernando Earthquake, 1625 Olympic Boulevard, Los Angeles	San Fernando Barthquake, 205 West Broadway, Long Beach	San Fernando Earthquake, Terminal Island, Long Beach	San Fernando Earthquake, Hall of Records, San Bernardino	San Fernando Barthquake, Fairmont Reservoir, Fairmont	San Fernando Barthquake, University of California, Santa Barbara	San Fernando Earthquake, Fire Station, Hemet
	File	No.	M.86	78.m	N1.88	1618	N196	361N	36 II	N197	8610	6610	7000	9050	9080	0207	9020	0210

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(Sheet 8 of 12)

	N/A	0.5	3.25	1.60	2.49	1.73	1.73	1.86	0.45	2.58	1.9	2.48	1.42	1.69	1.49	1.68		0.18
	us , in., for N/A	0.1	6.11	88.1	137.3	63.5	137.7	111	17.1	66.5	11.6	54.3	52.4	9.51	5.11	26.11		131
	, s	0.02	122.8	294.0	698.5	231.7	336.6	327.4	159.9	371.0	372.1	255.9	160.7	191.8	198.4	227.7		189.6
	Duration	0 40	15	35	99	58	58	35	30	36.85	30.00	36.86	25	50	23	28		50
	Modified	Intensity	VII	IIA	I,	Ľ.	ŗ.	>	15	VII	VII	H.	II,	II.	II,	II.	IIA	VII
	Richter	¥ 9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9	9.9
	Epicentral Distance	378.3	36.2	0.04	95.8	43.3	79.3	65.0	51.7	29.3	34.9	38.0	41.8	41.9	35.7	35.7	39.5	41.8
	A D	2.0	6.1	3.10	1.88.1	15.4 22.0 5.8	2.1.2	6.8 5.0 13.5	3.0	2.6.6	7.5	2.3	3.6	2.6 2.6	4.6.0	5.4.6	5.52	5.1
P. S. D. S.	Displace- ment	0.21 0.19 0.71	8.02 7.94 5.15	4.6 9.9 1.0	6.98 6.70 2.32	3.15 5.93 5.82	4.54 4.92 2.17	1.82	8.28 10.20 3.47	18.30 9.46 3.82	6.13 5.85 1.87	9.79 11.60 2.88	5.88	9.83	10.40	7.68 10.20 2.76	7.34 2.03	8.93 4.19 4.75
Α		0.27 0.29 0.59	23.20 16.20 9.84	14.70	7.01 5.78 3.47	5.29 6.66 1.16	7.25 5.51 3.19	4.50	10.60	31.50 17.80 9.65	13.6 10.30 7.49	17.20	17.90 19.60 8.73	18.30	16.70	19.70	16.20	16.70 18.70 7.78
¥	Peak Acceleration	0.65 0.86 0.86	154.00	108.00 88.10 60.10	24.10 34.30 9.29	137.00	25.90	53.20 37.80	41.30 37.70 17.90	243.00 197.00 96.00	167.00	119.00	138.00	149.00	106.00	184.00 174.00 88.90	79.80 81.10 57.30	195.00 188.00 67.50
	40	S 45° E S 45° E Up	S 89° W S 01° E Down	S 00° W N 90° E Down	S 000 N N 900 E Down	N 03° E N 87° W Down	300 s s s s s s s s s s s s s s s s s s	N 55° E N 35° W	N 000° E S 900° W Up	S 12° W N 78° W	South East Up	South East Up	N 37° E Up 53° ¥	N 53° W	South East Up	South East Up	3 145° E Up	3 53° E
	Spicenter	34°24'12" N	34°24'12" N 118°24'00" W	34°24'12" N	34°24'12" N	34°24.12" N	34°24'12" N	36 24.00" N	34°24'12" N 118°24'00" W	34°24.12" N	34°24.12" N	34° 24' 12" W	34°24'12" N	34°24.12" N 118°24.00" W	34°24'12" N	34°24°12" N 118°24°00" W	34°24.12" N	34°24'12" N 118°24'00" W
	Date	Sarthquake 2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71	2-9-71
	Site Classifi-	HB	H	æ	ы	esi Se	«E	95	4	æ	et.	ez,	**	A or I	4	æ	4	A or I
		San Fernando Earthquake, 1215 Gallery, Hoover Dam	San Fernando Barthquake, 1967 Sunset Boulevard, Los Angeles	San Fernando Earthquake, 3345 Wilshire Boulevard, Los Angeles	San Fernando Earthquake, 666 West 19th Street, Costa Mesa	San Fernando Earthquake, Santa Anita Reservoir, Arcadia	San Fernando Earthquake, Navy Laboratory, Fort Hueneme	San Fernando Earthquake. Puddingstone Reservoir. San Dimes	San Fernando Earthquake, 9841 Airport Boulevard, Los Angeles	San Fernando Earthquake. 14724 Ventura Boulevard. Los Angeles	San Fernando Earthquake, 1760 North Orchid Avenue, Los Angeles	San Fernando Earthquake, 9100 Wilshire Boulevard, Los Angeles	San Pernando Earthquake, 800 West First Street, Los Angeles	San Fernando Earthquake, 222 Figueroa Street, Los Angeles	San Pernando Parthquake, 6464 Sunset Boulevard, Los Angeles	San Fernando Earthquake, 6430 Sunset Boulevard, Los Angeles	San Fernando Earthquake, 1900 Avenue of the Stars, Los Angeles	San Fernando Barthouske, 231 South Figueros Street, Los Angeles
	File	0213	1224	7217	5550	P221	2222	F223	1533	4233	9630	6536	4211	15 A	9576	9218	6128	1522

								0								
CIL		Si.te				Peak	Peak	Peak Displace-		Epicentral	Richter	Modified				
F11e	Recording Station	Cation cation	Sarthquake	Epicenter	Instrument	Acceleration cm/sec2	Velocity cm/sec	ment cm	al cy	Distance	Magnitude M	Mercalli Intensity	Duration	0.05	us , in., for N/A	N/A
8523	San Fernando Earthquake, 533 South Fremont Avenue, Los Angeles	«	2-9-71	34,54,45" N	3 500 × 100 000 000 000 000 000 000 000 000	242.00 220.00 81.60	19.20 18.00 9.88	333	2.5	42.0	9.9	VII	52	280.2	64.5	1.1
\$500	San Fernando Earthquake, 6200 Wilshire Boulevard, Los Angeles	н	17-6-9	34°24.42" N 118°24.00" W	N 98° E N 82° W	123.00	22.50 21.90 5.20	15.80	8.6.3	38.9	9.9	VII	22	206.4	61.5	0.69
R	Sam Fernando Earthquake, 3440 University Avenue, Los Angeles	*	2-9-71	34°24'12" K 118°24'00" ¥	N 29° E S 61° E Up	25.30 24.50 24.50 24.50	17.20	10.30	3.6.6	9.44	9.9	VII				
1926	San Fernando Earthquake, 1177 Beverly Drive, Los Angeles	44	2-6-2	34°24°12" N 118°24°00" W	N 59° E N 31° W	197.70	18.30	12.20 5.92 2.26	5.90	39.6	9.9	IIV	39	2007	56.5	0.83
3262	San Fernando Earthquake, 5900 Wilshire Boulevard, Los Angeles		2-9-71	34°24,42" N 118°24.00" ¥	N 83° W S O7° W	8.8.8	25.70 27.80 6.17	16.50	2.1	39.0	9.9	IIA	20.23	100.8	1.50	1.46
5992	San Fernando Eartnquake, 3411 Wilshire Boulevard, Jos Angeles	H	12-6-2	34°24'12" ¥	South West Up	104.00	17.80 18.20 6.79	3.56	F F 2.9	39.9	9.9	TIA.	12	2007	ıäı	0.43
3566	San Fernando Earthquake, 3550 Wilshire Boulevard, Los Angeles	*	12-6-2	34°24'12" N	Morth West Up	153.00	21.50	8.04 11.60 3.15	9 6 6	0.04	9.9	IIA	90	175.6	1 60 1	0.74
5267	San Fernando Barthquake, 5260 Century Boulevard, Los Angeles	ef.	12-6-2	34°24'12" % 118°24'00" ¥	North East Up	25.58	28.8	8.19 9.38 3.61	3.1	52.0	9.9	Ľ.	9.6	218.9	58.6	0.70
1286	El Centro, Imperial Valley Irrigation District	4	19-21-17	32°58'00" N	North East Up	58.40 46.50 25.10	6.22	4.24 3.33 0.79	7.55	46.5	6.5	ıv	30	531.4	141.5	1.27
1287	El Centro, Imperial Valley Irrigation District	*	1-23-51	32°59'00" N 115°44'00" W	North East Up	30.30	3.09	1.95	9.9	27.5	5.6	Ŗ	8	323.2	0.61	2.79
1288	Zi Centro, Imperial Valley Irrigation District	40	6-13-53	32°57'00" N 115°43'00" W	North East Up	7.21 35.80 16.80	6.39	11.11	6.8 1.4 21.3	23.6	5.5	>	8	234.2	131	2.35
1289	El Centro, Imperial Valley Irrigation District	*	11-12-54	31°30'00" N 116°00'00" W	North East Up	24.10 27.00 6.74	3.76	2.66	7:17	149.8	6.3	IV	99	201.8	151	3.46
1292	El Centro, Imperial Valley Irrigation District	4	12-16-55	33°00'00" N	North East Up	62.50 71.00 56.40	5.16	2.06	5.8	23.5	5.4	Į,	30	310.1	121	4.27
1293	El Centro, Imperial Valley Irrigation District	4	8-7-66	31°48'00" ¥	North East Up	13.50	2.43 2.40 1.36	2.92	9.24	148.1	6.3	I,				
1620	City Hall, Perndale	н	7-6-34	41°42'00" N	N 450 W S 450 W	14.50 14.60 5.98	1.05	1.12	16.7	128.9		ħ				
0295	Federal Building, Helena, Montana	85	10-31-35	111°58'00" ¥	North East Up	29.30	0.54 0.39 0.52	0.32	32.1 26.5 17.6	80,		MI				
U297	Helena, Montana, Federal Building	HB	11-28-35	111°58'00" ¥	North East Up	74.80 83.00 31.70	3.22	0.99	5.5	8.8	5.0	17	50	310.7	50.6	0.72
						(e	(Continued)									

(Sheet 9 of 12)

	101		9	æ2	o.	0	5	-	0	-		0	C4		e .			9		
or N/A	0		1.96	0.18	2.39	1.90	0.95	4.81	0.09	2.11	1.81	00.00	3.52	4.74	0.63	0.94		3.36		of 12)
u , in., for N/A	0.1		12.7	93.4	6.58	83.8	11 62.0	18.5	39.8	76.5	85.7	181	154.2	87.6	31.7			114.6		(Sheet 10 of 12)
3	0.02		40.8	366.5	170.1	284.9	212.6	568.9	177.5	242.3	319.6	108.4	553.2	293.8	213.2	160.2		149.2		(3)
	Duration		15	30	90	33	35	59	30	30	55	35	09	90	#18	50		98		
Modified	Mercalli	>	VIII	VII	VII	TA.	I.	ĭ,	VII	VIII	ш	I.	>	IIA	VIII	14	ĭ,	I A	>	
Richter	Magnitude		6.6	4.9	5.3	5.3	5.0	5.7	5.7	6.5	5.6	5.8	5.2	6.3	6.3	4.5	5.4	9.9	3.8	
Epicentral	Distance	85.1	35.9	29.8	29.3	36.2	8.5	60.3	40.0	22.3	130.5	30.6	39.0	54.9	27.2	6.2	28.5	76.1	16.2	
A	14	5.48	13.4	6.6.4.6	3.0	9.5	3.5	7.2 6.7	3.4	6.00 A	9.4.	2.73	10.9	4.28	5.0 6.7 8.1	12.7	2.2	# 50 m 9	6.9.9	
D Penk Displace-	ment	0.90	3.74	2.95 2.51 1.12	1.40	2.24 1.36 1.06	1.85	1.21	3.00	2.55 5.43 1.62	2.53 1.49 1.50	1.76	2.26 2.03	8.21 16.30 5.72	22.70 11.80 26.30	3.56	0.85	0.80 1.26 1.20	0.32	
V	velocity cm/sec	4.07 2.71 1.59	21.70 21.60 3.64	6.92 5.74 2.56	11.70 8.26 3.63	4.19 4.52 1.94	5.25 3.64 2.10	3.11	10.80 6.28 4.23	5.59 9.35 8.35	2.21	11.80	2.67	17.30 23.60 9.07	29.40 16.50 30.10	7.61 9.32 1.04	1.33	2.89	0.28	(Continued)
Peak	Acceleration cm/sec2	38.40 35.90 13.90	233.00 172.00 68.50	118.00	193.00	52.00 48.90 23.10	35.50	57.50 73.50 14.40	168.00 74.90 60.20	52.10 77.50 32.10	8.10 11.20 5.95	103.00 232.00 32.40	13.10	62.30 95.60 63.60	192.00 155.00 279.00	39.70 53.60 8.47	11.20	52.90 35.40 26.30	2.02	
	Component	N 45° W S 45° W	N 45° E S 45° E Up	N 45° W	N 89° W S 01° W Up	N 89° W S 01° W Up	N 89° W S 01° W Up	N 16° W S 44° W Up	N 89° W S 01° W Up	S 32° E S 58° ¥ Up	3 21° E S 69° E Up	N 46° K S 44° K Up	N 89° W S 01° W Up	N 39° E N 51° W Up	South West Up	North East Up	S 50° E S 40° ₩ Up	N 36° ¥ S 54° ¥ Up	N 45° E	
	Epicenter	125°15'00" W	34°22' N 119°35' W	124°36' W	37°06' N 121°18' W	36°48' W	36°47° %	124°53° W	36°30° N 121°18° W	122°18' W	35°57'18" % 120°29'54" W	40°30' N	37°00'36" ¥ 121°47'18" ¥	33°37° N 117°58° W	33°37' N 117°58' W	33°47' N 118°15' W	33°47'00" N 118°15'00" W	35°50' N 121°10' W	37°40° N 122°28° W	
Date	Sarthquake	2-6-37	6-30-41	10-3-41	3-9-49	4-25-54	1-19-60	09-5-9	4-8-61	1-29-65	99-12-9	12-10-67	12-18-67	3-10-33	3-10-33	11-14-41	11-14-41	11-21-52	3-22-57	
Site	cation	н	4	н	۷.	۷	4	н	₹	4	4	н	*	I,A	4	۹	4	н	*	
	Recording Station c	City Hall, Ferndale	Santa Barbara Courthouse	City Hall, Ferndale	Public Library, Rollister	Public Library, Rollister	Public Library, Rollister	City Hall, Ferndale	Public Library, Hollister	Federal Office Building, Seattle, Washington	Lincoln School Punnel, Taft	City Hall, Ferndale	Hollister	Los Angeles Subvay Terminal Subbasement	Public Utilities Building Long Beach	Public Utilities Building, Long Beach	Los Angeles Chamber of Commerce Basement	City Recreation Building, San Luis Obispo	Southern Pacific Building Basement, San Francisco (Foreshock)	
EI3	No.	U298	n299	0300	0301	1305	U307	U308	60£0	0310	0311	U312	0313	V314	V315	V316	V317	V319	V320	

(Sheet 11 of 12)

	(A)	0.5			0.84		1.67	0.52		3.84			4.25	1.97	2.09	0.92	74.0	0.57
	u, , in., for N/A	0.1			34.7		28.7	15.5		37.2			11.1	8; 1 1 8; 1 1	83.6	15.7	35.0	102.5
	, n	0.02			173.1		189.2	57.9		155.2			197.3	131.8	324.7	136.7	212.8	286.5
	-	Sec			50		07	30		17			52	35	24	72	81	82
	Modified	Intensity	>	>	>	M	IA	>	IA.	IA	1,	VI	IA	Ľ	>	>	Þ	^
	Richter	M M	4.4	7.7	0.4	t4	5.0	6.0	6.3	4.5	5.4	4.6	5.4	5.4	5.4	5.4	4.9	4.9
	Epicentral	km	17.3	15.60	18.30	4.5	19.0	21.2	151.5	13.4	20.8	23.8	22.9	31.5	26.0	58.9	146.2	173.1
		27	5.0	13.9	2004	3.50	6.2	7.8 11.8 14.0	7.97	3.9	28.6	0.5.0	8.8 9.9 23.6	7.1	14.3	14.0 14.3 6.43	7.3	4.86.5
Desk	Displace-	cm	0.40	0.26	0.38	4.02 2.61 0.48	1.70	0.87	0.75	2.21	2.42 2.00	0.78 1.21 0.36	1.75	0.95	1.74	2.37	2.11	3.47 2.85 1.94
	Peak		0.83 2.61 0.88	0.98 0.98 98 98	0.42	17.90 8.85 1.93	3.52 2.67 1.50	2.12	1.57	8.87 9.63 3.18	2.56	2.94 3.96 1.25	4.75 3.10 1.85	1.87	1.53	2.00	3.53 2.71 1.80	1,38 2.21 2.21
4	Peak	cm/sec2	8.56 24.50 6.05	15.60 18.50 5.80	2.07 9.00 2.79	163.00 86.80 24.70	45.30 47.30 12.90	40.40 35.90 26.20	12.40	139.00	69.80 54.90 59.30	55.90 69.40 36.90	113.00 57.50 52.50	40.20 35.30 33.60	19.30 18.70 12.30	14.40 24.10 15.40	28.10	13.10 11.70 5.65
		0 1	N 45° S N 45° W	N 81° E N 09° W Up	N 45° E N 45° W	South West Up	N 79° E S 11° E Up	South East Down	South East Up	S 65° E S 25° W Down	S 85° E S 05° ₩ Down	S 54° E S 36° ₩ Down	North East Down	South East Up	North East Down	S 82° E S 08° W Down	South East Up	S 04° E S 26° ¥ Up
	The County or	Location	37°39'00" N 122°27'00" W	37°39'00" N 122°27'00" W	37°39' N 122°29' W	34°07'06" N 119°13'12" ¥	124°12' W	34°29'06" N 118°31'18" W	39°24'00" N	34°16'12" N 117°32'24" W	34°16'12" N 117°32'24" W	34°16'12" N 117°32'24" W	34°16'12" N 117°32'24" W	34°16'12" N 117°32'24" W	34°16'12" N 117°32'24" ¥	34°16'12" N 117°32'24" W	33°11'24" N 116°07'42" ¥	33°11'24" N 116°07'42" ¥
	Date	Earthquake	3-22-57	3-22-57	3-22-57	3-16-57	9-4-62	7-15-65	9-12-66	9-12-70	9-12-70	9-12-70	9-12-70	9-12-70	9-12-70	9-12-70	4-8-68	4-8-68
	Site	cation	4	н	«	4	н	H	∢	I	Æ	14	4	«	4	H	4	4
		Recording Station	San Francisco. South Pacific Building	San Francisco, Alexander Building	Southern Pacific Building Basement, San Francisco (Aftershock)	Port Hueneme	Federal Building, Sureka	Old Ridge Route (CWR Site), Castiac	Sacramento, Pacific Telephone and Telegraph	6074 Park Drive, Wrightwood	Cedar Springs, Allen Ranch	Cedar Springs, Pump House on dam abutment	Hall of Records, San Bernardino	Southern California Edison Company, Colton	Millikan Library Basement, CIT, Pasadena	J. P. L. Basement, Pasadena	Southern California Edison Company, Colton	Engineering Building, Santa Anna, Orange County
	CIT	No.	V322	V323	V328	V329	V330	V331	V332	#33¢	433 5	#336	4 338	4339	*34c*	4364	r370	13/11

Table B1 (Concluded)

	1								
N/A	0.5	2.60	2.81	1.96			0.36	1.34	2.19
fn., for	0.1	50.5	16.1	17.1			1 86 1	19.8	18.5
3	0.05	5.191	263.7 76.1 2.81	210.9			187.9	245.8	231.1
Duration	Sec	52	30	52 2			30	9	52
Modified Mercalli	Intensity	IA	M	ı	I,	ı	IA.	1.4	14
Richter	×	6.4	4.9	4.9	7.9	4.9	4.9	4.9	4.9
Epicentral Distance	Ę	205.1	220.3	212.9	212.0		218.8	212.2	227.3
A D	Ve	3.53	3.6	4.8.5	2.5	8.5.6	4 0 E	2.0.1	4.13
D Pesk Displace- ment	E S	4.98 2.11 1.82	0.53	1.70	2.02	1.98 2.31 1.36	2.30	2.50	2.12
V Peak Velocity	cm/sec	3.19 2.86 1.75	1.35	2.20	2.10 2.45 0.99	2.33 3.08 1.33	2.23 3.07 1.23	4.27	2.42
A Feak Acceleration	cm/sec2	8.73 9.51 5.14	7.35	9.82 10.30 6.38	6.99 10.00 3.81	1.90	6.97 11.40 5.41	18.40	10.90 12.30 4.79
Instrument	Component	м 21° м S 69° м Up	S 82° E S 08° E Down	North East Down	South West Up	N 52° W S 38° W Up	S 52° E S 38° W Up	N 83° W S 07° W Up	South East Up
Spicenter	Location	33°11'24" N 116°07'42" W	33°11'24" N 116°07'42" W	33°11'24" N 116°07'42" W	33°11'24" N 116°07'42" W	33°11'24" N 116°07'42" W	33°11'24" N 116°07'42" W	33°11'24" N 116°07"42" W	33°11°24" N 116°07'42" W
Date	Earthquake	4-8-68	4-8-68	4-8-68	4-8-68	4-8-68	4-8-68	4-8-68	4-8-68
Site Classifi-	cation	*	I,4	۷.	۷ .	4	Α,Ι	٧.	4
;	Recording Station	Terminal Island, Southern California Edison Plant, Long Beach	J. P. L. Basement, Pasadena	Millikan Basement, CIT, Pasadena	Pasadena, CIT Athenaeum	Southern California Edison Building, Los Angeles	Subway Terminal Basement. Los Angeles	CMD Building, Vernon	Hollywood Storage P. E. Lot, Los Angeles
Hi.	No.	1372	¥373	¥375	376	X377	1378	1379	1380

APPENDIX C: SYNTHETIC EARTHQUAKE RECORDS

Table Cl Synthetic Earthquake Records

Simulated Earthquake Type	Approximate Magnitude	A Maximum Acceleration cm/sec ²	V Maximum Velocity cm/sec	D Maximum Displacement cm	A2 AD	V ²	Approximate Predominant Period sec	Total Duration sec
CIT ⁴ A-1	**	382.77	58.99	39.83	4.38	0.228	0.50	120
A-2	**	441.64	55.05	72.97	10.63	0.094	0.35	120
B-1	7	368.12	45.72	33.17	5.84	0.171	0.20	50
B-2	7	308.70	48.26	22.22	2.94	0.339	0.22	50
0-1	9	66.93	6.65	1.36	5.06	0.486	0.15	12
C-2	9	57.23	60.9	0.88	1.36	0.736	0.20	12
D-1	5	470.40	26.67	4.88	3.23	0.310	0.15	10
D-2	5	490.00	28.94	6.84	7.00	0.245	0.15	10
Seed-Idriss	8-1/4	412.21	57.76	1	1	1	0,40	73

APPENDIX D: NOTATION

- A Maximum ground acceleration as a fraction of g
- D Maximum displacement
- g Acceleration of gravity
- M Earthquake magnitude
- N Ground acceleration, as a fraction of g, required to make factor of safety unity
- P Resultant of normal stress on slip surface
- p Normal stress on slip surface
- S Resultant of shear stress on slip surface
- s Shear stress on slip surface
- Subscript s Scaled value
 - t Time
 - t Time at cessation of relative motion
 - t Time at end of acceleration pulse
 - $\mathbf{u}_{\mathtt{m}}$ Displacement of sliding mass relative to ground
 - us Standardized maximum displacement; i.e., scaled permanent displacement of sliding mass for A = 0.5 and V = 30 in./sec
 - v Velocity
 - v_b Instantaneous velocity of sliding mass
 - vg Instantaneous ground velocity at time t
 - V Maximum ground velocity
 - W Weight of sliding mass
 - β Inclination of the resultant of shearing resistance, S, with respect to horizontal
 - θ Inclination of critical earthquake acceleration to horizontal

In accordance with letter from DAEN-RDC, DAEN-ASI dated 22 July 1977, Subject: Facsimile Catalog Cards for Laboratory Technical Publications, a facsimile catalog card in Library of Congress MARC format is reproduced below.

Franklin, Arley G

Earthquake resistance of earth and rock-fill dams; Report 5: Permanent displacements of earth embankments by Newmark sliding block analysis / by Arley G. Franklin, Frank K. Chang. Vicksburg, Miss.: U. S. Waterways Experiment Station; Springfield, Va.: available from National Technical Information Service, 1977.

38, 1217 p.: ill.; 27 cm. (Miscellaneous paper - U. S. Army Engineer Waterways Experiment Station; S-71-17, Report 5) Prepared for Office, Chief of Engineers, U. S. Army, Washington, D. C., under CWIS No. 31144.

References: p. 38.

1. Dam stability. 2. Displacement. 3. Earth dams. 4. Earth-quake engineering. 5. Earthquake resistant structures.

I. United States. Army. Corps of Engineers. II. Series:
United States. Waterways Experiment Station, Vicksburg, Miss.
Miscellaneous paper; S-71-17, Report 5.
TA7.W34m no.S-71-17 Report 5